



DEER MEADOWS PRELIMINARY PLAT STORM DRAINAGE CALCULATIONS

Prepared for:

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Project Location:

418 SW 12th St
College Place, WA
Parcel 350735420131

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Project #21-098



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STORM DRAINAGE CALCULATIONS

Deer Meadows
418 SW 12th St, College Place, WA
NW 1/4, SE 1/4, S15, T7N, R35E, WM

Project: New 11-lot subdivision with associated roads

Objective: Determine volume of stormwater runoff for the storage and infiltration system.

Method: Washington State Department of Ecology Stormwater Management Manual for Eastern Washington, February 2019,
Publication Number 18-10-044

Design Storms: 2-year 3-hour storm for construction stormwater
6-month 24-hour storm for volume-based treatment facility sizing.
6-month 3-hour storm for flow-based treatment facility sizing
25-year 24-hour storm for runoff storage volume and infiltration
25-year 3-hour storm for max runoff flow rate
100-year storms for Underground Injection Control (UIC) wells (drywells and infiltration trenches with perforated pipe)
100-year 72-hour regional storm for runoff storage volume and infiltration
100-year 3-hour storm for runoff storage volume and infiltration

Pollutant Loading

The new streets have an ADT of less than 7,500.

Pollutant Loading Classification = Low Table 5.22

Rainfall for Design Storms:

Precipitation depths are determined from precipitation charts for eastern Washington with adjustments for the 3-hour and 72-hour storms.

City = College Place
Region = 3
Annual Rainfall = 14-16 in

$P_{\text{yr}} \text{ 3hr} = 1.06 * C_{\text{sds}} * P_{\text{2hr2yr}}$	Eq. 4.2 to convert 2yr 2hr storm to 3hr storm for selected return period
$C_{\text{sds}} = 1$	Not used for 2yr storm.
$P_{\text{2yr 2hr}} = 0.45 \text{ in}$	Figure 4.6 for 2yr 2hr storm
$P_{\text{2yr 3hr}} = 0.477 \text{ in}$	Total Precipitation for 2yr 3hr storm
$C_{\text{sds 6mo}} = 0.66$	Table 4.7
$P_{\text{6mo 3hr}} = 0.31 \text{ in}$	Total Precipitation for 6mo 3hr storm
$C_{\text{sds 25yr}} = 1.9$	Table 4.7
$P_{\text{25yr 3hr}} = 0.91 \text{ in}$	Total Precipitation for 25yr 3hr storm
$C_{\text{sds 100yr}} = 2.66$	Table 4.7
$P_{\text{100yr 3hr}} = 1.27 \text{ in}$	Total Precipitation for 100yr 3hr storm
$P_{\text{2yr 24hr}} = 1.05 \text{ in}$	Figure 4.7
$C_{\text{wqs}} = 0.69$	Table 4.5, coefficient to compute 6mo 24hr from 2yr 24hr storm
$P_{\text{6mo 24hr}} = 0.72 \text{ in}$	
$P_{\text{25yr 24hr}} = 1.8 \text{ in}$	Figure 4.10 for 25yr 24hr storm
$P_{\text{100yr 24hr}} = 2.2 \text{ in}$	Figure 4.12 for 100yr 24hr storm
Regional Storm Factor (RSF) = 1.06	Table 4.6
$P_{\text{100yr 72hr}} = P_{\text{100yr 24hr}} * \text{RSF}$	Conversion from 24hr to 72hr regional storm
$P_{\text{100yr 72hr}} = 2.332 \text{ in}$	

STORM DRAINAGE CALCULATIONS

Deer Meadows
418 SW 12th St, College Place, WA
NW 1/4, SE 1/4, S15, T7N, R35E, WM

Summary of Soils Information

Soils information is from the Custom Soil Resource Report, provided by the United States Department of Agriculture Natural Resources Conservation Service and a Geotechnical Report prepared by PBS Dated June 25, 2018.

NRCS Soil Survey

Soil Types = Catherine Silt Loam

Hydrologic Soil Group = B

Saturated Hydraulic Conductivity, Ksat = .57 to 1.98 in/hr - per NRCS soils report

Geotechnical Report

Soil Description = Silt over poorly graded gravel

	TP-1	TP-2	Average Rate
Infiltration test =	2.00	1.5	in/hr
	Trench/Drywell	Swale	
Ksat vertical =	0.44	0.44	in/hr
Ksat horizontal =	0.88		in/hr
Cation Exchange Capacity =	>5	meq/100g	(Assumed based on silty soil)
Organic Content =	>1	%	(Assumed based on silty soil)
Treatment capacity rating =	High		Table 5.21: Vadose Zone Treatment Capacity

Well Logs

Material = Clay

Depth to Restrictive Layer = 33 ft

Depth to Groundwater = 12 ft

UIC Separation

Minimum thickness = 5 ft

UIC Separation Requirement Met ? Yes

Runoff Depth for Design Storms:

SCS curve numbers (CN's) from Table 4.14 are used to determine runoff depths for design storms. These values are based on antecedent moisture condition (AMC) II, with rainfall limits given in Table 4.15 as between 0.5 and 1.1 inches of rain (Dormant Season) in the 5 days prior to the design storm. For 100-year 72-hour storms, the Modified SCS Type 1A design storm requires an adjustment to CN's for AMC's I and III. While Region 2 normally has dry conditions (AMC I) due to low precipitation events, the more conservative CN values for average conditions (AMC II) are used instead. For regions with AMC III, an adjustment is needed to increase CN's for the regional storm.

Antecedent Precipitation = 27% Table 4.4

Antecedent Precipitation = 0.62964 in Compare value to the Total 5-Day Antecedent Rainfall for the Dormant Season, Table 4.15, to determine the AMC.

AMC = II If AMC I or II, use the CN values directly from Table 4.14 which are based on AMC II. If AMC III, then adjust the CN's for the 72hr regional storm per Table 4.16.

Total Runoff Depth (D) = $(P - 0.2 * S)^2 / (P + 0.8 * S)$

Eq. 4.20 to estimate runoff using the SCS curve number method.

D = 0 if P<0.2*S

S = $(1000 / CN) - 10$

	Paved	Gravel	Landscaped	
CN =	98	85	61	Runoff Curve Number based on soil type and surface cover under average antecedent moisture conditions, Table 4.14.
S =	0.204	1.765	6.393	
D2yr 3hr =	0.297	0.008	0.000	Depth of runoff during 2yr 3hr storm
D6mo 3hr =	0.157	0.000	0.000	Depth of runoff during 6mo 3hr storm
D25yr 3hr =	0.700	0.132	0.000	Depth of runoff during 25yr 3hr storm
D100yr 3hr =	1.053	0.313	0.000	Depth of runoff during 100yr 3hr storm
D2yr 24hr =	0.839	0.197	0.000	Depth of runoff during 2yr 24hr storm
D6mo 24hr =	0.527	0.065	0.000	Depth of runoff during 6mo 24hr storm
D25yr 24hr =	1.576	0.652	0.039	Depth of runoff during 25yr 24hr storm
D100yr 72hr =	2.104	1.046	0.149	Depth of runoff during 100yr 24hr storm

STORM DRAINAGE CALCULATIONS

Deer Meadows
418 SW 12th St, College Place, WA
NW 1/4, SE 1/4, S15, T7N, R35E, WM

Drainage Areas:

Areas 1 - 3 include the right of way plus 25 ft into the properties assumed to be pervious except for a 25ftx40 driveway for each lot. Area 4 is a 3,600 sf residence on each site, refer to attached drainage area exhibit.

		<u>Area 1</u>	<u>Area 2</u>	<u>Area 3</u>	<u>Areas 1-3</u>	<u>Area 4</u>
Pervious/Landscaped (sf)		4,025	19,505	5,470	29,000	0
Gravel (sf)		0	0	0	0	0
Impervious/Paved (sf)		10,305	42,495	9,980	62,780	3,600
Total Area (sf)		14,330	62,000	15,450	91,780	3,600
Total Area (acres)		0.33	1.42	0.35	2.11	0.08
Percent Impervious		72%	69%	65%	68%	100%
Pervious	CN ¹ =	61	61	61	61	85
	S =	6.393	6.393	6.393	6.393	1.765
Impervious	CN =	98	98	98	98	98
	S =	0.204	0.204	0.204	0.204	0.204

¹ - Pervious CN value is a weighted average between landscaping and gravel based on respective areas of each.

Runoff Volume & Peak Flow:

The peak flow rate is determined using the Santa Barbara Urban Hydrograph Method. The analyses for the 24-hour storms use the Type 1A design storm hyetograph (Table 4.31); the 3-hour analyses use the short duration storm hyetograph (Table 4.33). The 100-year 72-hour regional storm is modeled using the 24-hour Type 1A storm hyetograph with modifications for the regional storm factor and CN as applicable.

$$\text{Volume of Runoff} = (\text{D imp} * \text{Imp Area}) + (\text{D grvl} * \text{Grvl Area}) + (\text{D Indscp} * \text{Lndscp Area})$$

	<u>Area 1</u>	<u>Area 1</u>	<u>Area 2</u>	<u>Area 2</u>	<u>Area 3</u>	<u>Area 3</u>	<u>Areas 1-3</u>	<u>Areas 1-3</u>
	Peak Flow	Volume	Peak Flow	Volume	Peak Flow	Volume	Peak Flow	Volume
	(cfs)	(cf)	(cfs)	(cf)	(cfs)	(cf)	(cfs)	(cf)
6mo 3hr =	0.09	135	0.38	556	0.09	131	0.57	822
6mo 24hr =	0.03	452	0.13	1,865	0.03	438	0.19	2,755
2yr 3hr =	0.18	255	0.72	1,052	0.17	247	1.07	1,555
2yr 24hr =	0.05	721	0.21	2,973	0.05	698	0.31	4,392
25yr 3hr =	0.40	601	1.66	2,480	0.39	582	2.46	3,664
25yr 24hr =	0.10	1,367	0.40	5,646	0.09	1,329	0.58	8,342
100yr 3hr =	0.60	904	2.46	3,729	0.58	876	3.63	5,509
100yr 72hr =	0.13	1,857	0.52	7,692	0.12	1,818	0.77	11,366
	<u>Area 4</u>	<u>Area 4</u>						
	Peak Flow	Volume						
6mo 3hr =	0.03	47						
6mo 24hr =	0.01	158						
2yr 3hr =	0.06	89						
2yr 24hr =	0.02	252						
25yr 3hr =	0.14	210						
25yr 24hr =	0.03	473						
100yr 3hr =	0.21	316						
100yr 72hr =	0.04	631						

STORM DRAINAGE CALCULATIONS

Deer Meadows
418 SW 12th St, College Place, WA
NW 1/4, SE 1/4, S15, T7N, R35E, WM

Storage & Infiltration System

Runoff is directed to a swale in the southeast corner of the site. Storage volume required is calculated using the Santa Barbara Urban Hydrograph Method for the Type 1A design storm. The volume of the swale is calculated to the rim of the overflow manhole, and to the maximum water depth (1 ft below the top of the swale). When the pond fills to the level of the overflow manhole, runoff will be directed to an infiltration trench for below-ground infiltration. For the future buildings, the runoff will be directed to private infiltration trenches.

Trench/Swale	<u>Areas 1-3</u>	<u>Areas 1-3</u>	<u>Areas 1-3</u>	<u>Areas 1-3</u>	<u>Area 4</u>
Type =	swale	swale	trench	combined	trench
Bottom Length (ft) =			65		24
Bottom Width (ft) =			6		6
Bottom Area, A_{bot} (sf) =	897	897	390		144
Side Slope (H:V)=	2.5	2.5	0		0
Side Slope (H:V)=	2.5	2.5	0		0
Depth (ft) =	2.0	5.0	5.0		4.0
Top Length (ft) =			65		24
Top Width (ft) =			6		6
Top Area, A_{top} (sf) =	1,867	3,755	390		144
Middle Area, A_{mid} (sf) =	1,350	2,149	390		144
Pipe Diameter (in) =	0.0	0.0	12.0		8.0
Pipe Area (sf) =	0.0	0.0	0.8		0.3
Rock Porosity ² =	1	1	0.35		0.35
Total Storage Volume ³ (cf) =	2,721	11,040	716	11,756	207
Below Ground Infil. Rate (cfs) =	0.0000	0.0000	0.0183	0.0183	0.0063
Above Ground Rate ⁴ (cfs) =	0.0140	0.0236	0.0000	0.0140	0.0000

² - Rock Porosity = 0.35 for trenches (rock-filled) and 1.0 for swales (not rock-filled)

³ - Volume formula for swales and trenches: $V = h / 6 * (A_{bot} + 4 * A_{mid} + A_{top})$, where A_{mid} is the surface area of the trench/swale at mid-depth. Areas for irregular shapes measured using CAD software.

⁴ - Above ground infiltration rate includes infiltration along the bottom and halfway up the sides of a swale.

Design Summary	<u>Areas 1-3</u>	<u>Area 4</u>
Length of swale/trench (ft) =	0	24
# of Drywells (ea) =	0	0
Total Storage Volume (cf) =	11,756	207
Infil. Rate - below ground (cfs) =	0.0183	0.0063
Infil. Rate - above ground (cfs) =	0.0140	0.0000
Infil. Rate - time to infiltrate (cfs) =	0.0323	0.0063

25yr3hr

Required Storage (cf) =	3,393	162
Available/Required =	3.5	1.3
Sufficient Storage ?	Yes	Yes
Time to Infiltrate (hr) =	31.5	9.2

25yr24hr

Required Storage (cf) =	6,145	99
Available/Required =	1.9	2.1
Sufficient Storage ?	Yes	Yes
Time to Infiltrate (hr) =	71.7	20.8

100yr3hr - UIC's only

Required Storage (cf) =	5,211	
Available/Required =	2.3	
Sufficient Storage ?	Yes	
Time to Infiltrate (hr) =	47.3	

100yr72hr - UIC's only

Required Storage (cf) =	9,114	
Available/Required =	1.3	
Sufficient Storage ?	Yes	
Time to Infiltrate (hr) =	97.7	

The proposed storage and infiltration facilities are sufficient to store and infiltrate the runoff from the design storms.

STORM DRAINAGE CALCULATIONS

Deer Meadows
418 SW 12th St, College Place, WA
NW 1/4, SE 1/4, S15, T7N, R35E, WM

Site Suitability Criteria - Infiltration Trenches

Criteria

SSC-1: Setback Criteria

SSC-2: Ground Water Protection Areas

SSC-3: High Vehicle Traffic Areas

SSC-4: Soil Infiltration Rate/Draw down Time

SSC-5: Depth to Bedrock, Grounwater Table, or Impermeable Layer

SSC-6: Soil Physical and Chemical Suitability for Treatment

SSC-7: Seepage Analysis and Control

SSC-8: Cold Climate and Impact of Roadway Deicing Chemicals

Road deicing chemicals will not affect potable water wells.

SSC-9: Previously Contaminated Soils or Unsuitable Soils

The site isn't contaminated or located upgradient of contaminated sites.

Compliance

swales and trenches are not within noted setback requirements for wells, septic tanks, drainfields, springs, etc. Slopes are not a concern.
Site will not cause a violation of the ground water standards.

Not a high use site.

Infiltration tests showed 1.5 to 2 in/hr, design uses the upper layer of soil for infiltration and treatment, lower layer is only used beyond the treatment design storm. System draws down in less than 72-hours.

Depth to restrictive layer or groundwater is more than 5 ft below the bottom of the swales or infiltration trenches.

Soils meet requirements for cation exchange capacity $\geq 5\text{ meq}/100\text{g}$ and organic content $\geq 1\%$ and are thicker than 18".

Seepage will not cause adverse effects.

Core Element Assessment

Core Elements

#1: Stormwater Site Plan

Applicability

New development: applies to entire project

Compliance

Stormwater Site plan provided with construction documents.

Appropriate BMP's will be used.

#2: Construction Pollution Prevention

#3: Source Control

Applies to entire project

Applies to entire project

Sources of contamination will be controlled in accordance with State and local codes.

#4: Preservation of Natural Drainage

New development: applies to entire project

Site will be graded to keep stormwater on-site, no changes to natural drainage of surrounding areas.

#5: Runoff Treatment

Applies to Work Area ($> 5000 \text{ sf}$).

The swale will provide treatment. The rim for the overflow manhole that drains to the infiltration trench will be above the volume needed for the water quality design storm.

#6: Flow Control

N/A, runoff contained on site

#7: Operation and Maintenance

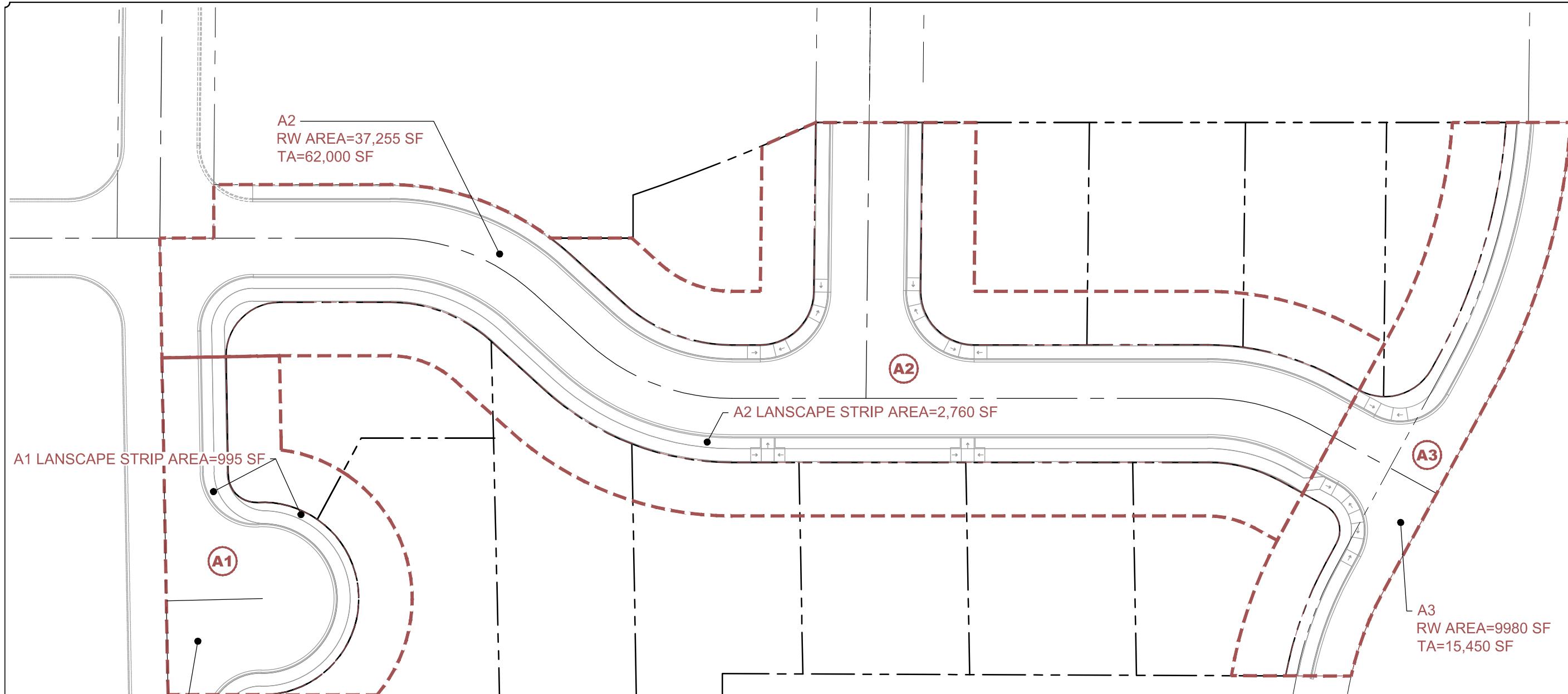
Applies to Cores #5 and #6

Catch basins, manholes, swale, and infiltration trenches will be regularly inspected and cleaned.

#8: Local Requirements

Applies to entire project

Site will be designed to meet local requirements.



50 25 0 50

PLAN SCALE

	A1	A2	A3	A1-3
IMPERVIOUS AREA:	10,305 SF	42,495 SF	9,980 SF	62,780 SF
GRAVEL AREA:	0 SF	0 SF	0 SF	0 SF
PERVIOUS AREA:	4,025 SF	19,505 SF	5,470 SF	29,000 SF
TOTAL AREA:	14,330 SF	62,000 SF	15,450 SF	91,780 SF

DEER MEADOWS STORM DRAIN AREAS

418 SW 12TH ST, COLLEGE PLACE, WA

J&J KELLY CONSTRUCTION
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Drawing Name	07-06-2022
Client/Project Information	Date
Project Number	21-098.1
Sheet Number	EX1B

HARMS
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File Path: P:\2021\14881\& Kelly Davis Ave Subdivision D.G.
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United States
Department of
Agriculture

NRCS

Natural
Resources
Conservation
Service

A product of the National Cooperative Soil Survey, a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local participants

Custom Soil Resource Report for Walla Walla County Area, Washington

Deer Meadows



Custom Soil Resource Report

Soil Map



Walla Walla County Area, Washington

AmA—Ahtanum silt loam, 0 to 3 percent slopes

Map Unit Setting

National map unit symbol: 2d5h

Elevation: 700 to 3,000 feet

Mean annual precipitation: 6 to 12 inches

Mean annual air temperature: 48 to 52 degrees F

Frost-free period: 140 to 165 days

Farmland classification: Not prime farmland

Map Unit Composition

Ahtanum and similar soils: 100 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Ahtanum

Setting

Landform: Depressions

Parent material: Loess and pumice alluvium

Typical profile

H1 - 0 to 7 inches: silt loam

H2 - 7 to 34 inches: silt loam

H3 - 34 to 40 inches: cemented silt loam

H4 - 40 to 60 inches: silt loam

Properties and qualities

Slope: 0 to 3 percent

Depth to restrictive feature: 20 to 40 inches to duripan

Drainage class: Somewhat poorly drained

Capacity of the most limiting layer to transmit water (Ksat): Very low to moderately high (0.00 to 0.57 in/hr)

Depth to water table: About 0 to 18 inches

Frequency of flooding: FrequentNone

Frequency of ponding: None

Calcium carbonate, maximum content: 10 percent

Maximum salinity: Moderately saline to strongly saline (8.0 to 16.0 mmhos/cm)

Sodium adsorption ratio, maximum: 10.0

Available water supply, 0 to 60 inches: Low (about 3.4 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 6w

Hydrologic Soil Group: C/D

Ecological site: R007XY401WA - ALKALI BOTTOM 6-10 PZ

Hydric soil rating: Yes

CaA—Catherine silt loam, 0 to 3 percent slopes

Map Unit Setting

National map unit symbol: 2d61

Elevation: 600 to 4,000 feet

Mean annual precipitation: 11 to 23 inches

Mean annual air temperature: 45 to 54 degrees F

Frost-free period: 100 to 195 days

Farmland classification: Prime farmland if drained

Map Unit Composition

Catherine and similar soils: 100 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Catherine

Setting

Landform: Valley floors

Parent material: Loess alluvium

Typical profile

H1 - 0 to 13 inches: silt loam

H2 - 13 to 48 inches: silt loam

H3 - 48 to 60 inches: stratified loamy sand to very gravelly silt loam

Properties and qualities

Slope: 0 to 3 percent

Depth to restrictive feature: More than 80 inches

Drainage class: Somewhat poorly drained

Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high
(0.57 to 1.98 in/hr)

Depth to water table: About 36 to 72 inches

Frequency of flooding: OccasionalNone

Frequency of ponding: None

Calcium carbonate, maximum content: 10 percent

Available water supply, 0 to 60 inches: High (about 11.3 inches)

Interpretive groups

Land capability classification (irrigated): 2w

Land capability classification (nonirrigated): 3w

Hydrologic Soil Group: B

Ecological site: R008XY601WA - WET MEADOW 10-16 PZ

Hydric soil rating: No

STATE OF WASHINGTON
 DEPARTMENT OF CONSERVATION
 AND DEVELOPMENT Appl. #6883
 Permit #6509

WELL LOG

No.

Date April 15, 1964

Record by Driller

Source Driller's Record

Location: State of WASHINGTON

County Walla Walla

Area 1446' E & 1426' N of S⁴

Map corner

NE 1/4 SE 1/4 sec. 35 T. 7 N., R. 35 E. E. W.

Diagram of Section

Drilling Co. Ross Davis Drilling

Address 1006 S. College Place, Washington

Method of Drilling Cable Date March 9, 1964

Owner Evern Budd

Address P.O. Box 116, College Place, Washington

Land surface, datum ft above
below

CORRELATION	MATERIAL	THICKNESS (feet)	DEPTH (feet)
-------------	----------	---------------------	-----------------

(Transcribe driller's terminology literally but paraphrase as necessary, in parentheses. If material water-bearing, so state and record static level if reported. Give depths in feet below land-surface datum unless otherwise indicated. Correlate with stratigraphic column, if feasible. Following log of materials, list all casings, perforations, screens, etc.)

Irrigation well - 6" x 44'			
Hardpan	0	8	
Gravel	8	12	
Sand, fine, brown	12	33	
Clay, light brown	33	35	
Sand, fine brown	35	40	
Sand and gravel	40	44	
Casing: 6" from 0 to 44'			
SWL: 12'			
Bailed 33 gpm with 2' DD after 1 hour			
Pump: 1 HP centrifugal, jet in well			

Turn up

Sheet of sheets

Table 4.5: Values of Coefficient C_{wqs} for Computing 6-Month, 24-Hour Precipitation

Climate Region Number	Climate Region Name	C_{wqs}
1	East Slope Cascades	0.70
2	Central Basin	0.66
3	Okanogan, Spokane, Palouse	0.69
4	Northeastern & Blue Mountains	0.70

Note: Values of C_{wqs} are based on the generalized extreme value (GEV) distribution whose distribution parameters can be expressed as a function of mean annual precipitation for eastern Washington.

Table 4.6: Factors for Converting From 24-Hour to Regional Storm Precipitation Depth

Climate Region Number	Climate Region Name	Multiplication Factor for Converting From 24-Hour to Regional Storm Precipitation Depth
1	East Slope Cascades	1.16
2	Central Basin	1.00
3	Okanogan, Spokane, Palouse	1.06
4	Northeastern & Blue Mountains	1.07

4.3.8 Precipitation Magnitude and Frequency for Short-Duration Storms

Design of flow-rate-based treatment BMPs using the single-event hydrograph method requires a determination of the 6-month, 3-hour precipitation depth for use with the 3-hour short-duration design storm hyetograph. (The updated design storm is indexed to sum to unity at 3 hours, so the 3-hour precipitation depth is needed to scale the hyetograph.) Design of other BMPs or conveyance elements based on the short-duration storm may also require the conversion of the 2-year, 2-hour precipitation to a 3-hour precipitation depth for a different recurrence interval.

The isopluvial map that is used as the starting point for determining the design precipitation depth for a 3-hour short-duration storm is a 2-year, 2-hour precipitation isopluvial map ([Figure 4.6: 2-Year, 2-Hour Isopluvial Map](#)).

The following equation is used to determine 3-hour precipitation for a selected return period:

Equation 4.2: Short-Duration Storm

$$P_{sds} = 1.06 * C_{sds} * P_{2yr2hr}$$

where:

P_{sds} = 3-hour precipitation (inches) for a selected return period for the short-duration storm

1.06 = multiplier used for all climate regions to convert
x-year, 2-hour precipitation to x-year, 3-hour precipitation

C_{sds} = coefficient (from Table 4.3.6) for converting 2-year,
2-hour precipitation to x-year, 2-hour precipitation depth

P_{2yr2hr} = 2-year, 2-hour precipitation (inches) from [Figure 4.6: 2-Year, 2-Hour Isopluvial Map](#)

[Table 4.7: Values of the Coefficient \$C_{sds}\$ for Using 2-Year, 2-Hour Precipitation to Compute 2-Hour Precipitation for Selected Periods of Return](#) lists values of the coefficient C_{sds} for selected return periods for various magnitudes of mean annual precipitation. An isopluvial map of average annual precipitation is shown in [Figure 4.1: Average Annual Precipitation and Climate Regions](#) and can be used to determine the mean annual precipitation for the site.

Table 4.7: Values of the Coefficient C_{sds} for Using 2-Year, 2-Hour Precipitation to Compute 2-Hour Precipitation for Selected Periods of Return

Climate Region Number	Mean Annual Precipitation (inches)	6-Month	1-Year	10-Year	25-Year	50-Year	100-Year
2	6–8	0.61	0.79	1.63	2.17	2.68	3.29
	8–10	0.62	0.80	1.60	2.09	2.55	3.09
	10–12	0.64	0.81	1.56	2.02	2.44	2.92
2, 3	12–16	0.66	0.82	1.51	1.90	2.26	2.66
3	16–22	0.67	0.83	1.47	1.82	2.13	2.48
1, 4	22–28	0.69	0.84	1.43	1.74	2.01	2.31
	28–40	0.70	0.85	1.40	1.68	1.92	2.19
	40–60	0.72	0.86	1.36	1.61	1.82	2.05
	60–120	0.74	0.87	1.33	1.55	1.74	1.93

Notes

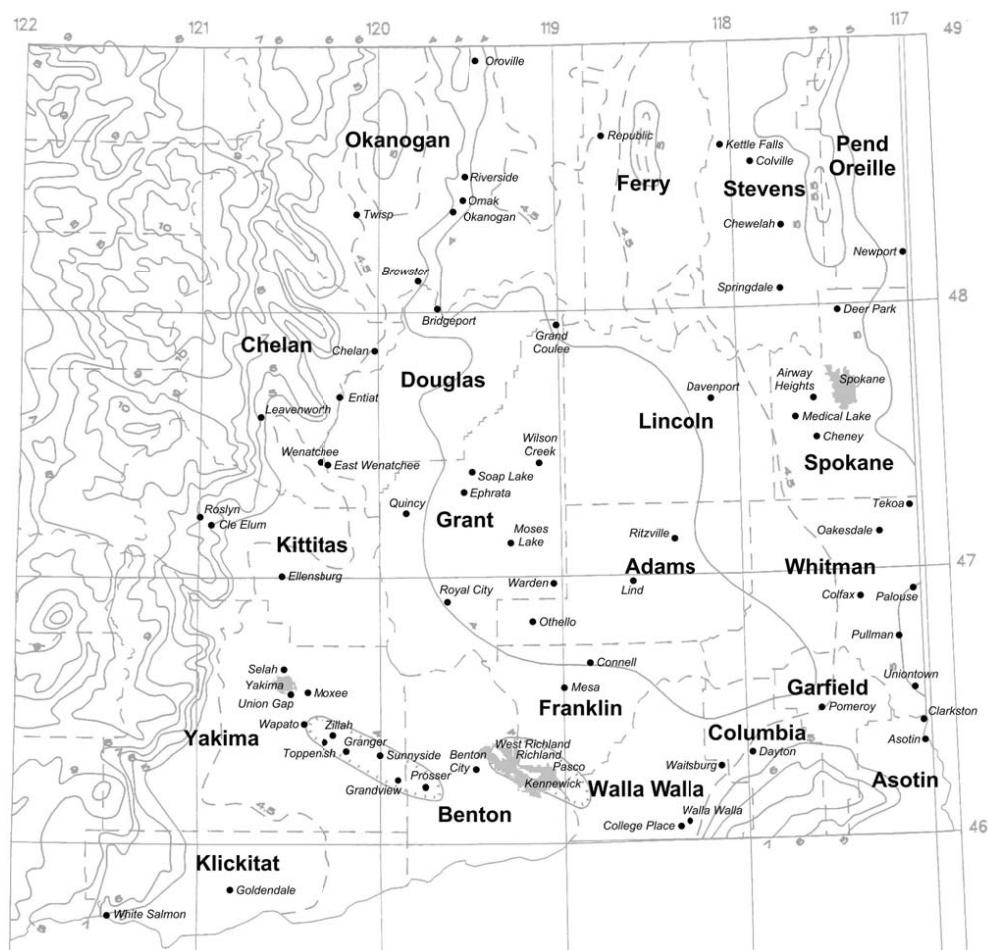
- The value for 2-hour precipitation is converted to 3-hour precipitation using a multiplier of 1.06 for all recurrence intervals.
- Values of C_{sds} are based on the generalized extreme value (GEV) distribution whose distribution parameters can be expressed as a function of mean annual precipitation for eastern Washington.

High ground water or shallow bedrock can cause a significant increase in runoff. If either of these conditions exists, it needs to be addressed by the designer. For a more complete discussion of computing weighted CN values, see *Urban Hydrology for Small Watersheds* ([USDA, 1986](#)).

Table 4.14: Runoff Curve Numbers (CNs) for Selected Agricultural, Suburban, and Urban Areas

Cover type and hydrologic condition	CNs for hydrologic soil group			
	A	B	C	D
Open space (lawns, parks, golf courses, cemeteries, landscaping, etc.)^a				
Poor condition (grass cover <50% of the area)	68	79	86	89
Fair condition (grass cover on 50% to 75% of the area)	49	69	79	84
Good condition (grass cover on >75% of the area)	39	61	74	80
Impervious areas				
Open water bodies: lakes, wetlands, ponds etc.	100	100	100	100
Paved parking lots, roofs, driveways, etc. (excluding right-of-way)	98	98	98	98
Permeable pavers and permeable interlocking concrete (assumed as 85% impervious and 15% lawn)				
Fair lawn condition (weighted average CNs)	95	96	97	97
Gravel (including right-of-way)	76	85	89	91
Dirt (including right-of-way)	72	82	87	89
Pasture, grassland, or range-continuous forage for grazing				
Poor condition (ground cover <50% or heavily grazed with no mulch)	68	79	86	89
Fair condition (ground cover 50% to 75% and not heavily grazed)	49	69	79	84
Good condition (ground cover >75% and lightly or only occasionally grazed)	39	61	74	80
Cultivated agricultural lands				
Row Crops (good) e.g., corn, sugar beets, soy beans	64	75	82	85
Small Grain (good) e.g., wheat, barley, flax	60	72	80	84
Meadow				
Continuous grass, protected from grazing and generally mowed for hay	30	58	71	78
Brush (brush-weed-grass mixture with brush the major element)				

Figure 4.6: 2-Year, 2-Hour Isopluvial Map



SOURCE: Dam Safety Guidelines, Technical Note 3, Design Storm Construction, Washington State Department of Ecology, Water Resources Program, report 92-55G April 1993.



2-Year, 2-Hour Isopluvial Map

Revised April 2002

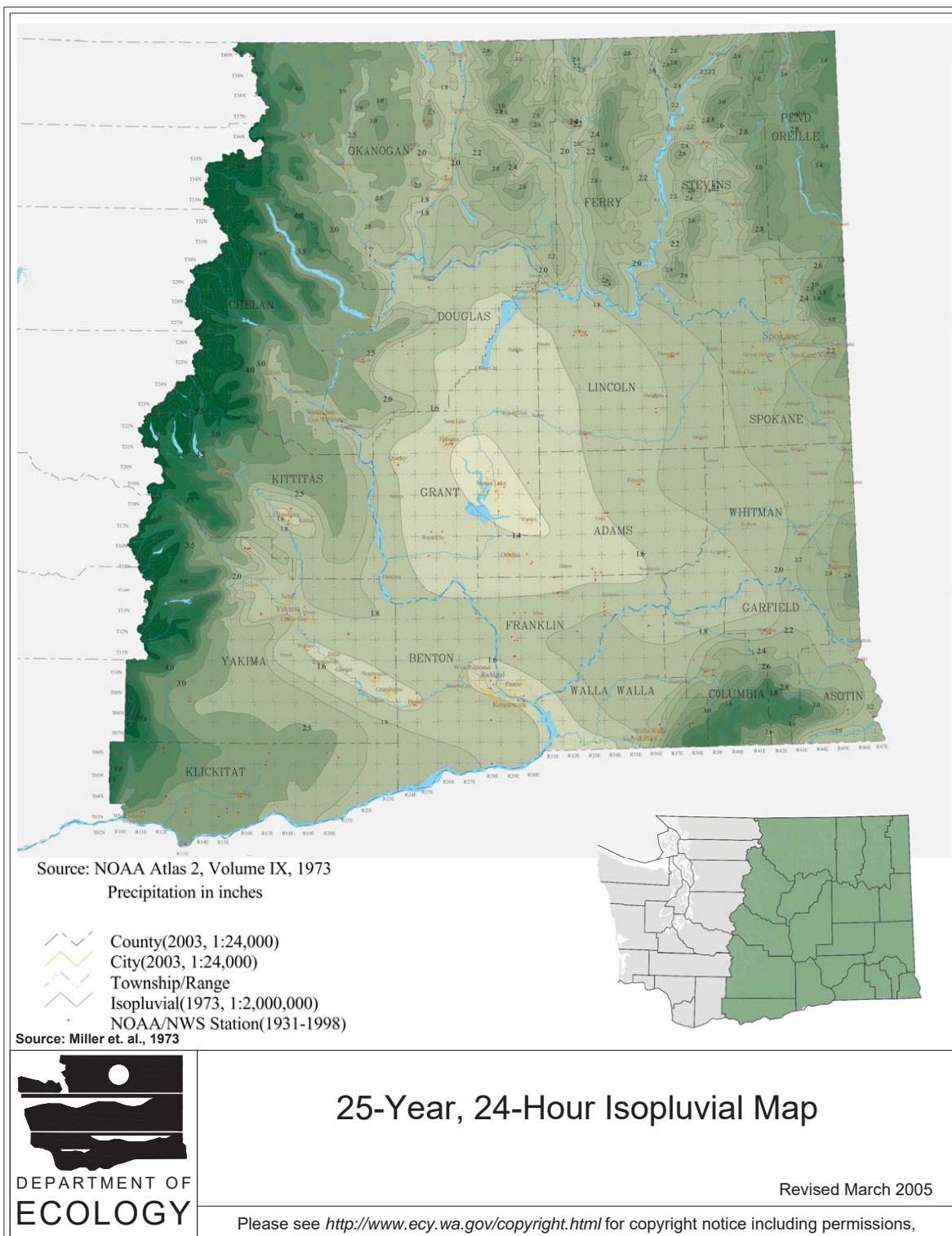
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College Place is very near the 0.45 in isopluvial.

Figure 4.7: 2-Year, 24-Hour Isopluvial Map

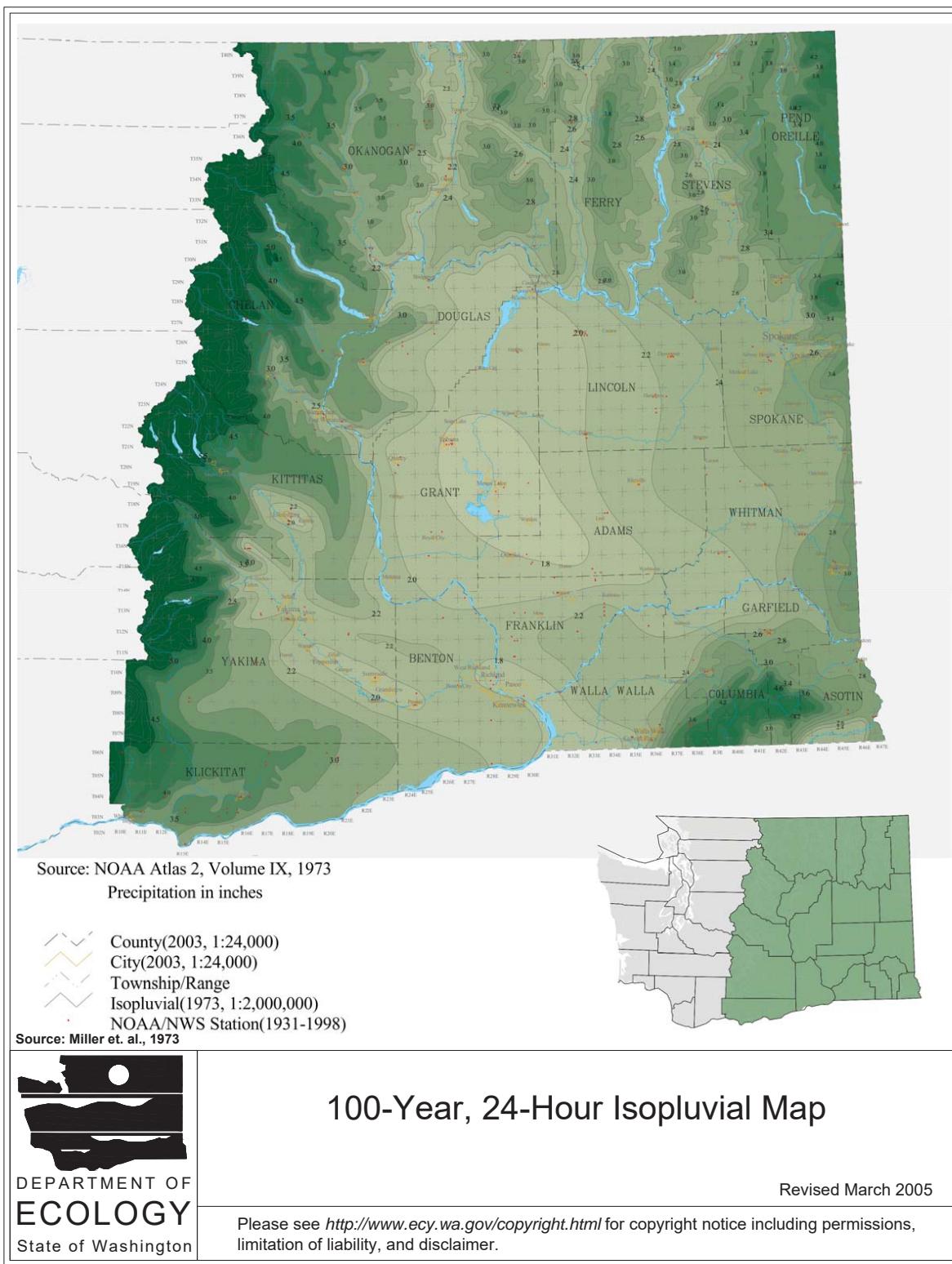


Figure 4.10: 25-Year, 24-Hour Isopluvial Map



College Place is between the 1.6 and 1.8" isopluvials, use 1.8"

Figure 4.12: 100-Year, 24-Hour Isopluvial Map



Areas 1-3

SCS Type 1A Storm Hyetograph Values

Santa Barbara Urban Hydrograph Method

Drainage Basin Area, A = 2.11 acres
 Total Rainfall, P25yr24hr = 1.8 inches
 Time Increment, dt = 6 minutes
 Time of Concentration, Tc = 5 minutes
 Below Ground Infil. Rate = 0.0183 cfs
 Above Ground Infil. Rate = 0.0140 cfs

Pervious Area: $A_{perv} = 0.67$ acres
 Impervious Area: $A_{imp} = 1.44$ acres

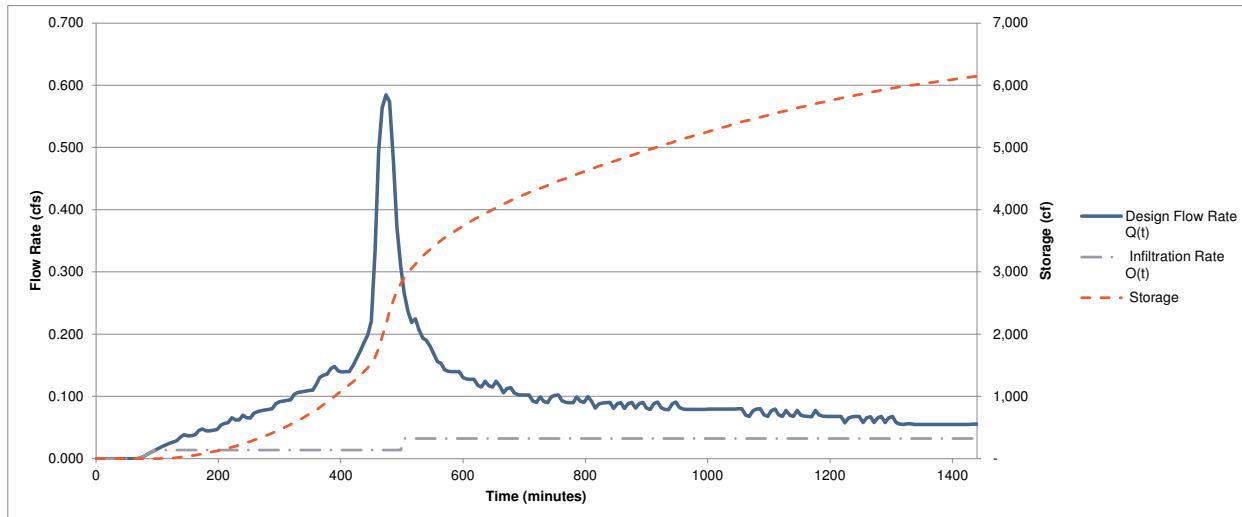
Abstract Runoff Value, S = $(1000/CN) - 10$
 Runoff Curve Number, CN = per Table 4.5.2
 Routing Constant, w = $dt/(2^*Tc + dt)$
 $w = 0.375$

Swale Volume to rim = 2,721 cf

Summary

Peak Design Flow =	0.584 cfs
Required Storage =	6,145 cf
Peak Hour Runoff =	1,517 cf

Storm: 25Yr 24Hr



Column Description and Formula

- 1 Time Step or Increment, t
- 2 Elapsed Time (minutes)
- 3 Rainfall Distribution, Type 1A Storm from Table 4.2.2
- 4 Incremental Rainfall, $P(t) = \text{Column 3} * P$
- 5 Accumulated Rainfall, $P = \text{Accumulated Sum of Column 4}$
- 6 Accumulated Runoff Depth Pervious Area If $P \leq 0.2^*S_{perv}$, then = 0; otherwise,
 $\text{If } P > 0.2^*S_{perv}, \text{ then } = (P - 0.2^*S_{perv})^{1/2}/(P + 0.8^*S_{perv})$
- 7 Incremental Runoff Depth, $D_{perv}(t) = \text{Column 6 of present step} - \text{Column 6 of previous step}$
- 8 Accumulated Runoff Depth Impervious Area If $P \leq 0.2^*S_{imp}$, then = 0; otherwise,
 $\text{If } P > 0.2^*S_{imp}, \text{ then } = (P - 0.2^*S_{imp})^{1/2}/(P + 0.8^*S_{imp})$
- 9 Incremental Runoff Depth, $D_{imp}(t) = \text{Column 7 of present step} - \text{Column 7 of previous step}$
- 10 Total Runoff Depth, $D(t) = (A_{perv}/A)*D_{perv}(t) + (A_{imp}/A)*D_{imp}(t)$
- 11 Instantaneous Hydrograph Flow Rate, $I(t) = 60.5^*D(t) * A/dt$
- 12 Design Flow Rate, $Q(t+1) = Q(t) + w^*[I(t) + I(t+1) - 2^*Q(t)]$
- 13 Accumulated Runoff = $Q(t)^*dt * 60 \text{ sec/min} + \text{Column 13 of previous step}$
- 14 Infiltration Rate = $(Ksat/(12^*3600)) * A$
- 15 Change in Storage = $dt * \{ [I(t) + I(t-1)]/2 - [(O(t) + O(t-1))/2]\}$
- 16 Storage = Storage from previous step plus change in storage

Areas 1-3															
Time Increment t	Elapsed Time (minutes)	Rainfall Distr. (fraction)	Incremental Rainfall P(t) (inches)	Accumulated Rainfall P (inches)	Accumulated Runoff D_perv(t) (inches)	Incremental Runoff D_perv(t) (inches)	Accumulated Runoff D_imp(t) (inches)	Incremental Runoff D_imp(t) (inches)	Total Runoff D(t) (inches)	Instant Flow Rate I(t) (cfs)	Design Flow Rate Q(t) (cfs)	Accumulated Runoff (cf)	Infiltration Rate O(t) (cfs)	Change in Storage (cf)	Storage (cf)
0	0	-	-	-	-	-	-	-	-	-	-	-	-	-	-
1	6	0.0020	0.004	0.004	-	-	-	-	-	-	-	-	-	-	-
2	12	0.0020	0.004	0.007	-	-	-	-	-	-	-	-	-	-	-
3	18	0.0020	0.004	0.011	-	-	-	-	-	-	-	-	-	-	-
4	24	0.0020	0.004	0.014	-	-	-	-	-	-	-	-	-	-	-
5	30	0.0020	0.004	0.018	-	-	-	-	-	-	-	-	-	-	-
6	36	0.0020	0.004	0.022	-	-	-	-	-	-	-	-	-	-	-
7	42	0.0020	0.004	0.025	-	-	-	-	-	-	-	-	-	-	-
8	48	0.0020	0.004	0.029	-	-	-	-	-	-	-	-	-	-	-
9	54	0.0020	0.004	0.032	-	-	-	-	-	-	-	-	-	-	-
10	60	0.0020	0.004	0.036	-	-	-	-	-	-	-	-	-	-	-
11	66	0.0030	0.005	0.041	-	-	0.000	0.000	0.000	0.00	0.000	0.0	0.000	-	-
12	72	0.0030	0.005	0.047	-	-	0.000	0.000	0.000	0.00	0.001	0.3	0.001	-	-
13	78	0.0030	0.005	0.052	-	-	0.001	0.000	0.000	0.01	0.004	1.6	0.004	-	-
14	84	0.0030	0.005	0.058	-	-	0.001	0.001	0.000	0.01	0.007	4.1	0.007	-	-
15	90	0.0030	0.005	0.063	-	-	0.002	0.001	0.001	0.01	0.010	7.8	0.010	-	-

Area 4

Storm: 25Yr 24Hr

SCS Type 1A Storm Hyetograph Values

Santa Barbara Urban Hydrograph Method

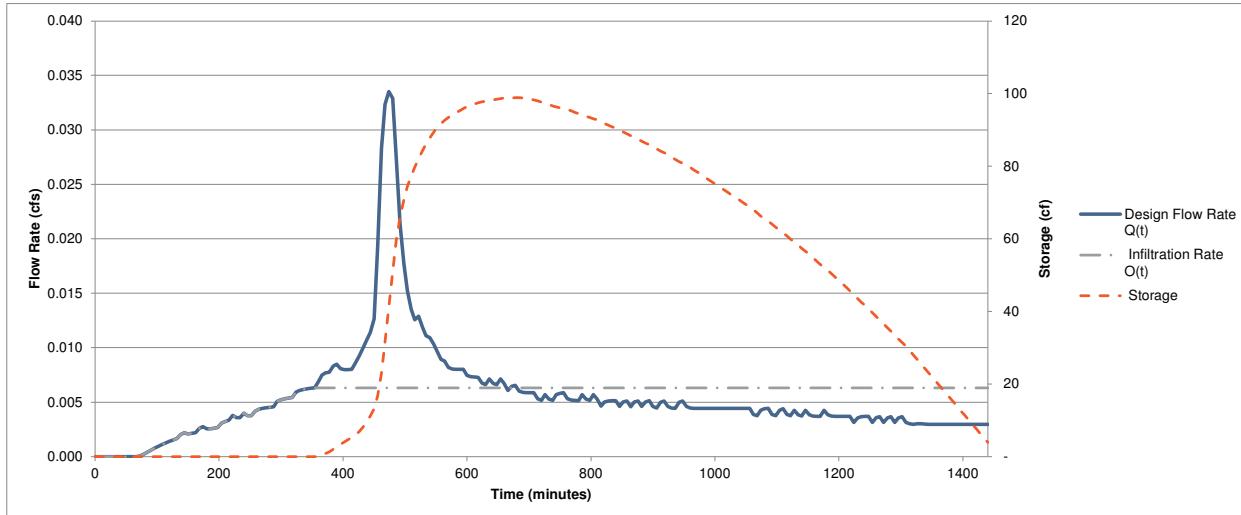
Drainage Basin Area, A = 0.08 acres
Total Rainfall, P25yr24hr = 1.8 inches
Time Increment, dt = 6 minutes
Time of Concentration, Tc = 5 minutes
Total Infiltration Rate = 0.0063 cfs

Abstract Runoff Value, S = $(1000/CN) - 10$
Runoff Curve Number, CN = per Table 4.5.2
Routing Constant, w = $dt/(2*Tc + dt)$
w = 0.375

Summary

Peak Design Flow =	0.034 cfs
Required Storage =	99 cf
Peak Hour Runoff =	87 cf

Pervious Area:	A _{perv} = 0.00 acres	CN _{perv} = 85	S _{perv} = 1.76	0.2*S _{perv} = 0.35
Impervious Area:	A _{imp} = 0.08 acres	CN _{imp} = 98	S _{imp} = 0.20	0.2*S _{imp} = 0.04



Column Description and Formula

- 1 Time Step or Increment, t
- 2 Elapsed Time (minutes)
- 3 Rainfall Distribution, Type 1A Storm from Table 4.2.2
- 4 Incremental Rainfall, P(t) = Column 3 * P
- 5 Accumulated Rainfall, P = Accumulated Sum of Column 4
- 6 Accumulated Runoff Depth Pervious Area If P ≤ 0.2*S_{perv}, then = 0; otherwise,
If P > 0.2*S_{perv}, then = $(P - 0.2*S_{perv})^{1/2}/(P + 0.8*S_{perv})$
- 7 Incremental Runoff Depth, D_{perv}(t) = Column 6 of present step - Column 6 of previous step
- 8 Accumulated Runoff Depth Impervious Area If P ≤ 0.2*S_{imp}, then = 0; otherwise,
If P > 0.2*S_{imp}, then = $(P - 0.2*S_{imp})^{1/2}/(P + 0.8*S_{imp})$
- 9 Incremental Runoff Depth, D_{imp}(t) = Column 7 of present step - Column 7 of previous step
- 10 Total Runoff Depth, D(t) = (A_{perv}/A)*D_{perv}(t) + (A_{imp}/A)*D_{imp}(t)
- 11 Instantaneous Hydrograph Flow Rate, I(t) = 60.5*D(t) * A/dt
- 12 Design Flow Rate, Q(t+1) = Q(t) + w*[I(t) + I(t+1) - 2*Q(t)]
- 13 Accumulated Runoff = Q(t)*dt*60 sec/min + Column 13 of previous step
- 14 Infiltration Rate = (Ksat/(12*3600)) * AI
- 15 Change in Storage = dt * { [I(t)+I(t-1)/2] - [(O(t)+O(t-1))/2] }
- 16 Storage = Storage from previous step plus change in storage

Area 4															
Time Increment t	Elapsed Time (minutes)	Rainfall Distr. (fraction)	Incremental Rainfall P(t) (inches)	Accumulated Rainfall P (inches)	Accumulated Runoff (inches)	Incremental Runoff D _{perv} (t) (inches)	Accumulated Runoff (inches)	Incremental Runoff D _{imp} (t) (inches)	Total Runoff D(t) (inches)	Instant Flow Rate I(t) (cfs)	Design Flow Rate Q(t) (cfs)	Accumulated Runoff (cf)	Infiltration Rate O(t) (cfs)	Change in Storage (cf)	Storage (cf)
0	0	-	-	-	-	-	-	-	-	-	-	-	-	-	-
1	6	0.0020	0.004	0.004	-	-	-	-	-	-	-	-	-	-	-
2	12	0.0020	0.004	0.007	-	-	-	-	-	-	-	-	-	-	-
3	18	0.0020	0.004	0.011	-	-	-	-	-	-	-	-	-	-	-
4	24	0.0020	0.004	0.014	-	-	-	-	-	-	-	-	-	-	-
5	30	0.0020	0.004	0.018	-	-	-	-	-	-	-	-	-	-	-
6	36	0.0020	0.004	0.022	-	-	-	-	-	-	-	-	-	-	-
7	42	0.0020	0.004	0.025	-	-	-	-	-	-	-	-	-	-	-
8	48	0.0020	0.004	0.029	-	-	-	-	-	-	-	-	-	-	-
9	54	0.0020	0.004	0.032	-	-	-	-	-	-	-	-	-	-	-
10	60	0.0020	0.004	0.036	-	-	-	-	-	-	-	-	-	-	-
11	66	0.0030	0.005	0.041	-	-	0.000	0.000	0.000	0.00	0.000	0.0	0.000	-	-
12	72	0.0030	0.005	0.047	-	-	0.000	0.000	0.000	0.00	0.000	0.0	0.000	-	-
13	78	0.0030	0.005	0.052	-	-	0.001	0.000	0.000	0.00	0.000	0.1	0.000	-	-
14	84	0.0030	0.005	0.058	-	-	0.001	0.001	0.001	0.00	0.000	0.2	0.000	-	-
15	90	0.0030	0.005	0.063	-	-	0.002	0.001	0.001	0.00	0.001	0.4	0.001	-	-

