SITE PLAN REVIEW FINAL APPLICATION

CONVENIENCE STORE AND OFFICE BUILDING

Tax Map 5, Lots 47C & 48-1 134 Main Street East Waterboro, Maine

June 19, 2019

Prepared For

NEW HORIZONS MANAGEMENT COMPANY, LLC 3391 White Sulphur Road Gainesville, GA 30501

Prepared By



119 Purinton Road, Suite A, Brunswick Landing, Brunswick, ME 04011 207-725-1200 • www.sitelinespa.com Site Plan Review Final Application Convenience Store and Office Building June 19, 2019

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June 19, 2019



3841-7

Mr. Lee Jay Feldman, Director of Planning Town of Waterboro 24 Townhouse Road East Waterboro, Maine 04030

Re: Site Plan Review Application <u>CONVENIENCE STORE AND OFFICE BUILDING</u> <u>134 MAIN STREET, EAST WATERBORO, MAINE</u> Tax Map 5, Lot 47C & 48-1

Dear Lee:

On behalf of New Horizons Management Company, LLC, Sitelines PA is submitting twelve (12) copies of a Site Plan Review Application and plans for the construction of a convenience store building with associated gasoline and diesel filling stations, an office building, and associated parking, infrastructure, and landscaping located at 134 Main Street. This letter is intended to summarize the project in order to facilitate the review process.

PROPERTY

New Horizons Management Company, LLC owns two (2) parcels of land located at 124 and 134 Main Street, respectively (Tax Map 5, Lots 47C & 48-1). The parcels contain approximately 2.61 acres and have frontage on Main Street (Route 202). The property at 134 Main Street is currently developed as a convenience store with gasoline and diesel fueling stations and the property at 124 Main Street is currently developed with a gravel access drive and a parking area. In total, the two properties contain approximately 35,916 s.f. (0.82 acres) of impervious area. The property is located in the Village Zoning District, in which Gas Stations, Retail and Service Stores, and Offices are permitted uses requiring lot sizes of 40,000 s.f. or greater.

PROJECT DESCRIPTION

The proposed project consists of the construction of two (2) buildings; a 6,380 s.f. convenience store, and an 1,826 s.f. office building. The project will be accessed from two (2) curb cuts from Main Street (U.S. Route 202).

In addition to the typical use of the convenience store, there will be a restaurant use within the building. As part of the development, seven (7) filling stations (5 gasoline, 1 diesel, and 1 K-1) will be constructed adjacent to the convenience store building. The existing filling stations on the property will be removed.

The existing building on the site is going to be relocated on the property and reutilized as an office building.

Site Plan Review Application Convenience Store and Office Building June 19, 2019 Page 2 of 7

In total, the project will create a total of approximately 55,191 s.f. (1.27 acres) of impervious area, or a net increase of 19,275 s.f. (0.44 acres) of impervious area.

As the project results in less than an acre of new impervious area, but more than an acre of disturbed area, a Stormwater Management Law Permit-by-Rule from the Maine Department of Environmental Protection (MDEP) will be required.

The parcel has been delineated for wetlands by Atlantic Environmental, LLC which are shown on the enclosed plans. As part of the proposed development, approximately 1,359 s.f. of wetlands will be impacted for the installation of the outfall from the stormwater system. As the proposed wetland impacts are less than 4,300 s.f., the project does not require a freshwater wetlands alteration permit from the MDEP.

SITE PLAN REVIEW STANDARDS

To facilitate your review of our proposal, the following issues are summarized in accordance with *§2.10 Site Plan Review Standards* of the Waterboro Code.

1. The proposed development meets the definitions and/or requirements set forth in the Zoning Ordinance;

The project has been designed to meet the standards as outlined within the Ordinance. The applicant is requesting some waivers from the Zoning Ordinance and those consist of the following:

- Stormwater Treatment Based on the Zoning Ordinance, the project requires treatment of 95% of the impervious area and 80% of the developed area. We request that the applicant provide stormwater treatment for the new impervious and developed areas created as a result of the proposed redevelopment.
- Groundwater Study Based on the Zoning Ordinance, a Groundwater Study is required to ensure that the proposed septic system does not result in any adverse impacts to any adjacent properties. The impacts to the abutting properties are not a concern as the wells located on the adjacent properties have been located and are more than 100-feet from the proposed septic system. Furthermore, the proposed septic system will be sized for less than 1,000 gallons per day, which is a relatively small commercial septic system that is similar in sizing to a residential system. A Well Setback Plan is provided within the application package.

2. The proposed development will not create fire safety hazards by not providing adequate access to the site, or to the buildings on the site, for emergency vehicles; or adequate fire suppression systems;

As the development has been designed to accommodate gas-delivery vehicles, it will also provide adequate access for emergency vehicles. A plan showing the truck turning movements throughout the site has been enclosed with this submission. The fire suppression system to be installed within the new filling station canopy will comply with all NFPA regulations.



3. The proposed exterior lighting will not create hazards to motorists traveling on adjacent public streets; be inadequate for the safety of occupants or users of the site or will damage the value and diminish the usability of adjacent properties;

A Lighting Plan showing the type of lighting and illumination levels for the site and the impacts to adjacent properties has been developed and is enclosed with this submission. Lighting will include full cut-off fixtures and house shields where necessary.

4. The provisions for buffers and on-site landscaping provides adequate protection to neighboring properties from detrimental features of the development;

A Landscape Plan, conforming to the Town's Ordinances, has been developed and has been enclosed with this submission. Due to the location of the property, and the layout of the development, the landscaping will be concentrated along the frontage of Main Street (U.S. Route 202).

5. The proposed development will not have a significant detrimental effect on the use and peaceful enjoyment of abutting property as a result of noise, vibrations, fumes, odor, dust, glare or other cause;

As the project consists of a convenience store with filling stations and an office building, there is no excessive noise, vibrations, fumes, odor, dust, or glare anticipated as a result of the development. The project is being constructed in an area of the Town that is zoned for commercial use and is replacing an existing convenience store with filling stations.

6. The provisions for vehicular loading and unloading and parking and for vehicular and pedestrian circulation on the site and onto adjacent public streets will not create hazards to safety or will not impose significant burdens on public facilities which could be avoided by reasonable modification of the plan;

Per the Ordinance, for a proposed convenience store, the use of "Retail shops and service store" appears to be most applicable. For that use, there is one (1) parking space required for each 200 sf of store area plus one (1) space for every three (3) employees. Within the convenience store, there is approximately 2,300 s.f. of "store area." The remainder of the store consists of bathrooms, a sitdown eating area, food preparation areas, storage areas, and cooler areas. The 2,300 s.f. of "store area" would require 11.5 parking spaces. Within the convenience store, there is also a sit-down area that consists of 24 seats. Per the Ordinance, for a restaurant use, there is one (1) parking space required for every three (3) seats plus one (1) space for every three (3) employees. The 24 seats would require 8 parking spaces. Based on discussions with the owner, there would be no more than four (4) employees on staff at one time, which would require an additional 1.3 parking spaces. In total, the convenience store would require 20.8 parking spaces. As proposed, there are currently 28 parking spaces allocated to the convenience store use.

Per the Ordinance, for a professional office, one (1) parking space is required for each 200 sf of non-storage floor area plus one (1) space for every three (3) employees. With the tenant of the building and internal floor plan not finalized, it is difficult to determine the exact parking demand.



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If there were no storage areas provided in the building, the 1,826 s.f. building would require 9.2 parking spaces. With 13 parking spaces allocated for the office building, that leaves four (4) parking spaces to be utilized for employee parking. Per the Ordinance requirements, that would permit up to twelve (12) employees in the office building, which seems reasonable given the size of the building.

7. The bulk, location, height or design of proposed buildings, structures or paved areas, or the proposed uses thereof, will not have a significant detrimental effect on private development on adjacent properties, or on the value of adjacent properties which could be avoided by reasonable modifications of the plan;

The building will incorporate materials and accents traditional to New England architecture. The building proportions, height and other dimensions, windows and features of the facade have been designed by a Maine registered architect. Architectural elevation drawings have been prepared by ALPHAarchitects and are included with this submission.

8. The design of the site will not result in significant flood hazards or flood damage and is in conformance with applicable flood hazard protection requirements;

The project area is in Zone C (Areas of Minimal Flooding) of the Flood Insurance Rate Maps (FIRMs) for Cumberland County, Maine. The project area is located on Panel 20 of 20 (Community Panel 230199-0020-C, Effective February 1, 1985). An excerpt of the applicable FIRM is enclosed with this submission.

9. Adequate provisions have been made for the disposal of wastewater or solid waste or for the prevention of ground or surface water contamination;

Wastewater from the two (2) buildings will be treated by a new private subsurface wastewater septic field. The existing septic field is not adequately sized to accommodate the demand from the new buildings. Kenneth Gardner, LSE, has completed three (3) test pits on the property which all provided suitable locations for the new septic field. He is currently working on the septic design, which will be submitted as part of the building permit process. The proposed septic field will be located more than 300 feet away from the existing public supply well on the property and more than 100 feet away from residential wells on adjacent properties. A Well Setback Plan has been enclosed with this submission illustrating the location of the septic test pits in relation to the adjacent public and private wells. The test pit logs for the septic locations have been enclosed with this submission.

Solid waste will be stored in an enclosed dumpster on site and collected by a licensed waste hauler.

10. Adequate provisions have been made to control erosion or sedimentation;

Temporary erosion control measures include silt fence and hay bale barriers to prevent silt from leaving the development site. Permanent erosion control measures will include seeding and mulching of disturbed areas immediately after final grading is completed. Erosion control measures will remain in place until the area has been properly stabilized. The project will use methods as



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outlined in the "Maine Erosion and Sediment Control Handbook for Construction: Best Management practices" by the Maine Department of Environmental Protection.

11. Adequate provisions have been made to handle storm water run-off or other drainage problems on the site and any proposed stormwater detention ponds are adequate to serve stormwater on or from the site;

The new impervious and developed areas created as a result of the redevelopment of the site will be directed to a grassed underdrained soil filter for treatment and detention prior to discharge to the large wetland complex located in the southwest corner of the property. A Stormwater Management Plan, which further details the stormwater treatment and detention system, has been developed for the project and has been enclosed with this submission.

12. The proposed water supply will meet the demands of the proposed use and is adequate for fire protection purposes;

The proposed development will utilize the existing well that currently serves the existing convenience store to serve the new convenience store and office building. As this well serves the public as part of the sit-down food service within the convenience store, it is classified as a public water supply. As such, the well requires periodic testing and requires a 300-foot setback from any septic systems. The existing septic system, and new fuel storage tanks are exempt from this setback as they are legally existing uses. The new septic system, which serves the new convenience store, is located outside of this 300-foot setback.

13. Adequate provisions have been made for the transportation, storage and disposal of hazardous substances and materials as defined by state law and the Hazardous Waste Ordinance;

The proposed gasoline, diesel, and K-1 filling stations and associated underground tanks will comply with all State and Federal Regulations concerning installation and maintenance. The existing tank will be removed as part of the project. Any soil contamination will be addressed at the time of the tank removal.

14. The proposed use will not have an adverse impact on significant scenic vistas or on significant wildlife habitat which could be avoided by reasonable modification of the plan;

There are no known scenic vistas or significant wildlife habitats within the proximity of the project site. The proposed use is not anticipated to have any adverse impact on any scenic vistas or significant wildlife habitat.

15. The project will not increase nitrate nitrogen concentrations in surface or groundwater at the property line of the site in excess of State of Maine Drinking Water Standards. If groundwater contains contaminants in excess of the primary drinking water standards and the project is to be served by on site groundwater supplies, the applicant shall demonstrate how water quality will be improved or treated to meet applicable standards.



Site Plan Review Application Convenience Store and Office Building June 19, 2019 Page 6 of 7

The project will be served by a new subsurface wastewater system and an existing well. The proposed subsurface wastewater system will be located the required 100-feet away from the any residential wells on adjacent properties. The applicant is requesting a waiver from a Groundwater Study as the proposed subsurface wastewater system will meet the requirements as outlined within the State of Maine Subsurface Wastewater Disposal Rules. The proposed septic system will be sized for less than 1,000 gallons per day, which is a relatively small commercial septic system that is not anticipated to have any adverse impacts on the adjacent properties.

COMMERCIAL DESIGN STANDARDS

To facilitate your review of our proposal, the following issues are summarized in accordance with *§4.12 Commercial Design Standards* of the Waterboro Code.

- 1. Proposed buildings, improvements and additions shall not be stylized to the point that the building or improvements are more an advertisement than an architectural form.
- 2. Long or continuous facades do not provide visual interest. Facades shall be articulated every 50 feet using varied designs, rooflines, materials and heights. The front facade shall be designed to look like more than one building entrance when the façade is proposed to be greater than 50 feet in width
- 3. The relationship of the width of the building to the height of the front elevation shall be visually compatible with buildings, structures and open spaces where it is visually related. A proposed new building or structure shall break up uninteresting box-like forms into smaller, varied masses comparable to a variety of form and massing which are often elements essential to the character of the streetscape. Avoid single, monolithic forms that are not relieved by variations in massing
- 4. The roof shape of a building shall be visually compatible with that of buildings to which it is visually related. When no clear pattern exists, a roof pitch of 5/12 or steeper shall be used, or the building should be designed so as to appear to have a pitched roof. The design of the roofline or parapet wall shall screen any air conditioning or other utilities placed on the roof on 3 sides most visible to the pub

The building will incorporate materials and accents traditional to New England architecture. The building proportions, height and other dimensions, windows and features of the facade have been designed by a Maine registered architect. Architectural elevation drawings have been prepared by ALPHAarchitects and are included with this submission.

5. Where mechanical equipment such as HVAC are located at ground level, appropriate vegetative screening shall be used to hide the equipment while spaced to allow for routine maintenance.

For the convenience store, the HVAC equipment will be roof mounted on the rear of the building and will screened. Refer to the enclosed architectural elevations. The HVAC equipment for the office building will be ground-mounted but will be located at the rear of the building and will be screened from the roadway.



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6. For trash receptacles located on the property, where possible, natural vegetative screening and or stockade fencing shall be used to screen the containers from public view.

As shown on the enclosed plans, the dumpster pad is located to the rear of the property and is enclosed with a chain link fence with slats.

SUMMARY

We trust that this information satisfactorily addresses the requirements for Site Plan Review, and we look forward to meeting with you and the Planning Board at their July 10, 2019 meeting to review the project and obtain their feedback. Should you have any questions or require any additional information, please contact me.

Very truly yours,

Joseph J. Marden, P.E. Project Manager

Enclosures

cc: Daniel Peck III, New Horizons Management Company, LLC Wes Thames, Priority Real Estate Group, LLC







Attachment A Application Form & Checklist

A completed copy of the Town of Waterboro Planning Board Agenda Application Form and the Checklist are enclosed.

TOWN OF WATERBORO PLACEMENT ON PLANNING BOARD AGENDA

Date received:		
APPLICATION TO BE PLACED ON PLANNING	BOARD AGENDA	
I,		
Address:	Tax Map #	Lot #
	Zone	
	Telephone #	
HEREBY MAKE APPLICATION TO BE PLACED PLANNING BOARD:	ON THE AGENDA OF TH	E WATERBORO
Nature of business to be presented before the board:		
	ANNO 11 1	*****
Is the project in the Shoreland Zone? yes	no	
Estimate of time necessary for presentation: 30 min	45 min	
Name (s) of person (s) who will be appearing before	the Planning Board:	
 Please file this form with the Code Enforcement Office You will be notified of the date and time you are to any the date, time and purpose of your meeting and all any concerns they may have in writing to the Plan Building permit application Diagram of the lot and project in relation to set of the properties of the pr	cer, at which time a non-refu opear. You shall notify all a low them 10 days prior to the ning Board. Attach the follow aid lot	Indable fee* must be paid. Abutters of your property of the meeting date to submit towing information:
WATERBORO PLANNING BOARD FEES* Placement on agenda (informational) Conditional use / set back reduction (includes relocation in Shoreland Zone)	N/A \$100.00	
Special Structures (Height modification) Cluster development	\$50.00 \$100.00 +\$50.00 for each o residential, commercial or in the development	development review - industrial unit

\$100.00 +\$50.00 for each development review residential, commercial or industrial unit

in the development

\$50.00

_____ Planned Unit

_____ Temporary Use Review

Rev 08162018

SITE PLAN REVIEW MASTER CHECKLIST TOWN OF WATERBORO

Site Plan Name:	Location:	Map
Lot		•

All applications for Site Plan review shall be filed with the Code Enforcement Officer and processed to the appropriate municipal reviews by the Town Planner.

Ten (10) copies of 24" \times 36" plans shall be drawn to scale of not greater than 1" to 100' and three (3) copies of the Site Plan in 11" \times 17" format showing the following features, both existing and proposed:

ITEM	ITEM	SITE PLAN DETAILS
SUBMITTED	REVIEWED	
	ВУ	
		Name of project, names and addresses of owners of record; tax map and lot
		number.
		North arrow, date of plat, scale; name, address and seal of person preparing the
		plan with an appropriate signature block
		Vicinity sketch and zoning district(s)
		Boundaries of the site and abutting streets with widths, including length of lot
		lines
		Footprint of building - showing the number of stories, dimensions of
		structure(s), accesses and use, including decks and outbuildings
		Layout and location of off-street parking and loading, access drives and
		vehicular maneuvering areas
		Location and size of all signs, gasoline pumps, and similar free standing
		structures
		Waste/dumpster locations and snow storage areas
		Location, direction and type of outdoor lighting
		Location and type of screening and/or buffers and other landscaping
		Location of all utilities
		Topography of a contour interval not greater than 2 ft. showing the effects
		upon adjacent property
		Hydrogeological impact study for any site where a septic system design flow is
		in excess of 800 gallons or if predominantly made up of non-typical septic waste.
		This study must contain components as listed in Section D.9a-f of Site Plan
		Ordinance.
		Meets definitions and requirements set forth in the Zoning Ordinance

ITEM	ITEM	SITE PLAN DETAILS
SUBMITTED	REVIEWED	
	ВУ	
		Provides adequate access to the site for emergency vehicles; access without
		parked cars in way
		Provides adequate dry hydrants and access to the hydrants and Fire
		Department sprinkler connection and pressure hydrants and/or cisterns, as
		applicable
		Proposed exterior lighting creates no hazard to motorists on adjacent streets
		or occupants of adjacent properties
		Provide a detailed buffer zone and on-site landscaping for protection of
		neighboring properties
		Proposed use will not disturb the peaceful enjoyment of abutting property
		owners as a result of noise, vibrations, fumes, odor, dust, glare or other cause.
		The provisions for parking and loading and pedestrian circulation on the site and
		adjacent streets will not create any safety hazard or impose significant burdens
		on public facilities which could be avoided by reasonable modifications of the
		plan. Parking will not prohibit access of emergency vehicles.
		The proposed use of the site or its buildings will have no significant effect on
		private development of adjacent properties, or the value of adjacent properties
		which could be avoided by reasonable modifications to the plan. Keep with
		character of neighborhood.
		The design of the site will not result in significant flood hazards or flood
		damage and is in conformance with applicable flood hazard requirements.
		The site contains an adequate storm water management plan.
		Adequate provisions are made for the disposal of wastewater and solid waste.
		Adequate provisions for drainage on site.
		The proposed water supply meets the demands of the use and/or for fire
		protection.
		Snow removal and adequate storage, so it will not obstruct parking or
		civilian/emergency vehicle traffic.
		Provisions for the storage of hazardous waste as defined by State law and
		Waterboro Hazardous Waste Ordinance. Hazardous materials proposed will be
		identified.
		The proposed use had no adverse impact on significant scenic vistas or on
		significant wildlife habitat which could be avoided by reasonable modification to
		the plan.
		A nitrate study has been submitted if the nitrogen concentration in surface or
		groundwater at the property line of the site is in excess of 5 mg/l.
		Demonstrate the treatment of the water supply if the groundwater contains
		contaminants in excess of primary drinking water standards and the project is
		to be served by on-site groundwater supplies.
		Pertormance bond as per section 12.04 of the Zoning Ordinance (see process
		below)
		Sprinklered or not with additional intrastructure
		Site walk date determined

ITEM SUBMITTED	ITEM REVIEWED BY	SITE PLAN DETAILS

	STATE AND FEDERAL REVIEWS
Letter Received	MDOT scoping session/traffic impact report
Letter Received	MDEP Site Location Review complete
Letter Received	State Fire Marshall
	LOCAL DEPARTMENT REVEIWS
Letter Received	Waterboro Water District (consultation)
Letter Received	Waterboro Fire Department
Letter Received	Waterboro Road Review Committee
Letter Received	Waterboro Code Enforcement Officer
Letter Received	Waterboro Town Planner
	PERFORMANCE BOND PROCESS
Letter Received	Calculated infrastructure costs (from applicant's engineer)
Letter Received	Municipal review and peer review of infrastructure costs
Letter Received	Anticipated build out schedule
Passbook Received	Savings Deposit Book to Town of Waterboro

WAIVER REQUEST FORM Town of Waterboro, Maine

If there is more than one waiver requested, each waiver request is to be individually listed and described, as each waiver is considered individually by the Town of Waterboro Planning Board. Each petition for waiver shall be submitted in writing by the applicant with the application (site plan or subdivision) for review. The request shall fully state the reasons for which the waiver is requested and any/all facts supporting the request. Additionally, each waiver that may be granted by the Planning Board, shall be listed on the approved site or subdivision plan.

Name of S	Name of Site/Subdivision Plan: Convenience Store and Office Building								
Site/Subdivision Location:134 Main Street, East Waterboro									
Map #:5		Lot#:X		_Zoning: _	V				
Owner(s)	New Horizon	s Managemer	nt Company, Ll	LC					
Address o	f Owners:33	91 White Sul	phur Road, Ga	inesville, GA	30501				
Phone #:	N/A	***	Email:	N/A					
Land Surve	eyor: X								
Phone #:			Email:						
Engineer:	Sitelines, PA A	ttn: Joseph J.	Marden, PE						
Phone #:	207-725-1200	Ext. 12	Email:	jmarden@s	itelinespa.com				
• • • • • • • •									

2.10.11 - Stormwater Management

Based on the Zoning Ordinance, the project requires treatment of 95% of the impervious area and 80% of the developed area. We request that the applicant provide stormwater treatment for the new impervious and developed areas created as a result of the proposed redevelopment. Per that standard, the project provides treatment of 182% of the new impervious area and 141% of the total new developed area.

Signature of Owner/Applicant

Review Date by Planning Board:_____

Planning Board Decision:__

Approved, Approved with Modifications, Denied (with reasons

WAIVER REQUEST FORM Town of Waterboro, Maine

If there is more than one waiver requested, each waiver request is to be individually listed and described, as each waiver is considered individually by the Town of Waterboro Planning Board. Each petition for waiver shall be submitted in writing by the applicant with the application (site plan or subdivision) for review. The request shall fully state the reasons for which the waiver is requested and any/all facts supporting the request. Additionally, each waiver that may be granted by the Planning Board, shall be listed on the approved site or subdivision plan.

Name of Site/Subdivision Plan: Convenience Store and Office Building									
Site/Subdivision Location:134 Main Street, East Waterboro									
Map #:_ ⁵	Lot#: <u>47C * 48-1</u> Zoning:V								
Owner(s) New Ho	rizons Management Company, LLC								
Address of Owners:	3391 White Sulphur Road, Gainesville, GA 30501								
Phone #:N/A	Email: <u>N/A</u>								
Land Surveyor:									
Phone #:	Email:								
Engineer: Sitelines,	PA Attn: Joseph J. Marden, PE								
Phone #:207-725-	1200 Ext. 12 Email: jmarden@sitelinespa.com								

Joseph Marden ______ seek the following described waiver to the Town of Waterboro: (Be specific and reference sections of Site Plan, Zoning or Subdivision Ordinances)

2.10.15 - Groundwater Study

Based on the Zoning Ordinance, a Groundwater Study is required to ensure that the proposed septic system does not result in any adverse impacts to any adjacent properties. The impacts to the abutting properties are not a concern as the wells located on the adjacent properties have been located and are more than 100-feet from the proposed septic system. Furthermore, the proposed septic system will be sized for less than 1,000 gallons per day, which is a relatively small commercial septic system that is similar in sizing to a residential system. A Well Setback Plan is provided within the application package.

 \aleph AGE NT

Date of submitta

Signature of Owner/Applicant

Review Date by Planning Board:_____

Planning Board Decision:___

Approved, Approved with Modifications, Denied (with reasons



June 13, 2019

3841-2

Mr. Daniel Peck III New Horizon Management Company, LLC 3391 White Sulphur Road Gainesville, GA 30501 <via email>

Re: Designation of Agent Authorization Convenience Store and Office Building 134 Main Street, East Waterboro, Maine Tax Map 5, Lots 47C & 48-1

Dear Daniel:

As required by various approval agencies, please indicate by signing below that Sitelines, PA is authorized to act as agent for New Horizon Management Company, LLC, for the specific purpose of preparation and submission of local and state permitting applications on your behalf for the proposed convenience store and office building to be located at 134 Main Street in East Waterboro, Maine.

Sincerely.

Joseph J. Marden, P.E. Project Manager

The undersigned hereby gives Sitelines, PA the authority to act as agent for New Horizons Management Company, LLC for the specific purpose of preparation and submission of local and state permitting applications for the project specifically identified above.

Daniel Peck II

Site Plan Review Final Application Convenience Store and Office Building June 19, 2019

<u>Attachment B</u> <u>Right, Title, & Interest</u>

A copy of the current deeds is included with this attachment.

WARRANTY DEED

KNOW ALL MEN BY THESE PRESENTS that **ABBOTT INVESTMENTS**, **LLC**, a Maine Limited Liability Company, whose mailing address is 29 Hamilton Road, Lyman, Maine 04002, for consideration paid hereby grants to **NEW HORIZON MANAGEMENT COMPANY**, **LLC**, a Georgia Limited Liability Company, whose mailing address is 3395 White Sulphur Road, Gainesville, GA 30501, with WARRANTY COVENANTS, the premises situated in the Town of Waterboro, County of York. and State of Maine, bounded and described as follows:

> SEE EXHIBIT "A" ATTACHED HERETO AND INCORPORATED HEREIN BY REFERENCE.

IN WITNESS WHEREOF, the said **ABBOTT INVESTMENTS**, **LLC**, has caused this instrument to be executed this 27th day of October, 2017.

Witness

ABBOTT INVESTMENTS, LLC

3555-5-6

By: David J. Abbott Its: Manager, duly authorized

STATE OF MAINE YORK, ss.

October 27, 2017

Then personally appeared the above-named **David J. Abbott in his capacity as Manager of Abbott Investments, LLC,** and acknowledged the foregoing instrument to be his free act and deed in his said capacity.

Before me. Notary TASDON Maine Notary Public dme/\server\Data\Lisa\Documents\Real\Abbott. David (134 Main Street. Waterboro - Sale Ad Avwo Goon mission Expires January 29, 2020

EXHIBIT "A"

A certain lot or parcel of land, together with buildings and improvements thereon, situated on the southerly side of Routes 4, 5, & 202, in the Town of Waterboro, County of York, State of Maine, and being more particularly bounded and described as follows:

Beginning at a point, being a set 3/4" iron rod on the assumed southerly sideline of Routes 4, 5, & 202, at the northwesterly corner of land now or formerly as described in the deed of Warren A. Eaton and Mary A. Eaton to Kenneth W. Eaton and Claire L. Eaton dated October 10, 1964, and recorded in the York County Registry of Deeds in Book 1622, Page 309, said point of beginning also being N 13° 51' 15" E, 1.85 feet, from a found iron pipe;

Thence N 80° 51' 15" W, along said Routes 4, 5, & 202, 200.40 feet, to a set 3/4" iron rod; thence S 14° 33' 15" W, along the remaining land now or formerly of Forrest M. Abbott, Jr. and Isabel O. Abbott, 205.16 feet, to a found 5/8" iron rod; thence S 26° 42' 45" E, along remaining land now or formerly of said Forrest M. Abbott, Jr. and Isabel O. Abbott, 170.00 feet, to a found 5/8" iron rod; thence S 75° 21' 05" E, along the remaining land now or formerly of said Forrest M. Abbott, 91.69 feet, to a set 3/4" iron rod at the land now or formerly of said Eaton, being N 01° 43' 30" E, 4.79 feet, from a found 1-1/4" iron pipe; thence N 13° 51' 15" E, along said Eaton land and remains of a stonewall, 352.00 feet to the point of beginning.

Containing 1.42 Acres, more or less.

The basis for this description is Grid North determined between the found Maine Department of Transportation railroad spikes in the pavement of station p.c. 124+36.15, and station p.c. 138+17.48 as shown on the Maine Department of Transportation Right of Way Map dated September 1980 and revised last May 10, 1981, and recorded in said Registry in Plan 140, Page 15. Said bearing is S 76° 55' 05" E.

This description is based from a "Standard Boundary Survey For Forrest M. Abbott, Jr." dated July 21, 1994, by Robert A. Yarumian, II, P.L.S. 1303 of Maine Boundary Consultants of West Buxton, Maine.

All said found 5/8" iron rods are rebar with a yellow plastic cap marked "PLS 2161".

All said set 3/4" iron rods are rebar with an aluminum cap marked "SET BY ROBERT A YARUMIAN PLS 1303 LAND SURVEY MONUMENT".

Title reference may be had to Warranty Deed from David J. Abbott and Judith A. Abbott to Abbott Investments, LLC, dated February 10, 2010 and recorded in the York County Registry of Deeds in Book 15815, Page 770.

dmc/\server\Data\Lisa\Documents\Real\Abbott, David (134 Main Street, Waterboro - Sale) 10.17\WD.doc

WARRANTY DEED

KNOW ALL MEN BY THESE PRESENTS that DOUGLAS F. ABBOTT and CONSTANCE J. ABBOTT of Zeffer, Florida, for consideration paid hereby grant to NEW HORIZON MANAGEMENT COMPANY, LLC, with a mailing address of 3395 White Sulphur Road, Gainsville, GA 30501, with WARRANTY COVENANTS, the premises situated in the Town of Waterboro, County of York and State of Maine, bounded and described as follows:

> SEE EXHIBIT "A" ATTACHED HERETO AND INCORPORATED HEREIN BY REFERENCE.

IN WITNESS WHEREOF, the said DOUGLAS F. ABBOTT and CONSTANCE J. ABBOTT have hereunto set their hands and seals this 5th day of October, 2018.

Witness

Witness

STATE OF MAINE YORK, ss.

DOUGLAS F. ABBOTT

CONSTANCE J. ABBOTT

October 5, 2018

Then personally appeared the above-named Douglas F. Abbott and Constance J. Abbott and acknowledged the foregoing instrument to be their free act and deed.

Before me,

Notary Public/Attorney-at-Law

STEPHEN Y. HODSDON nh/\server\Data\Lisa\Documents\Real\Peck, Daniel (Purchase of land from Abbott)\warranty deed

Maine Notary Public **Commission Expires** January 29, 2020

Portion of Tax Map 5; Lot 47C (124 Main Street, Waterboro)

A certain lot or parcel of land located on the southerly side of Rt. 202, aka Main Street, so called, in the Town of Waterboro, County of York and State of Maine, bounded and described as follows, to wit:

Beginning at a 1" iron pipe, marking the easterly corner of land conveyed to New Horizon Management Co. LLC as described in Deed Book 17591, Page 436, recorded in the York County Registry of Deeds;

Thence S 62° 29' E along said Rt. 202 and the land of these Grantors, a distance of 260.2 feet, to a 5/8" capped rebar and the land of Mary Young as described in Deed Book 3168, Page 177;

Thence S 40° 03' W along the land of said Young, and the land of Warren as described in Deed Book 15950, Page 761, a distance of 224.0 feet to a 5/8" capped rebar set;

Thence N 58° 13' W through the land of these Grantors, a distance of 229.2 feet to the easterly boundary of said New Horizon Management Co. LLC, and a 5/8" rebar set;

Thence N 32° 20' E along the land of said New Horizon Management Co. LLC, a distance of 202.3 feet to the point of beginning, containing 51,619 Square Feet.

Meaning to describe and hereby describing a portion of the land as described in a deed from Kenneth W. Eaton to Douglas F. Abbott and Constance J. Abbott in Book 7387, Page 226, recorded in said Registry of Deeds and shown on a Division Survey for Douglas F. Abbott, prepared by Stephen W. Everett, dated September 22, 2018, attached hereto.

Said premises are conveyed subject to Standard Easement Deed from the Grantors to Central Maine Power Company & Saco River Telephone Company dated August 28, 1997 and recorded in the York County Registry of Deeds in Book 8451, Page 350.

EXCEPTING & RESERVING to the Grantors, their heirs and assigns, a right of way for ingress and egress and an easement for installation, maintenance, repair and replacement of all utility services, including without limitation thereof, electricity, water, sewer, telephone & cable telephone services, for the Grantors' remaining property, on, over, under and across the area depicted on the attached Division Survey and labelled thereon as "gravel drive, access for Douglas Abbott."

nh/\server\Data\Lisa\Documents\Real\Peck, Daniel (Purchase of land from Abbott)\warranty deed to Peck.doc



Attachment C Abutting Property Owners

A copy of the abutters map and a list of abutting property owners are included in this attachment for reference.



<u>Abutters List</u> 134 Main Street, East Waterboro, Maine

Locus Properties

N/F

NEW HORIZON MANAGEMENT CO, LLC 3395 WHITE SULPHUR RD GAINESVILL, GA 30501 BK 17591, PG 438 MAP 5 LOT 48-1

N/F

NEW HORIZON MANAGEMENT CO, LLC 3395 WHITE SULPHUR RD GAINESVILL, GA 30501 MAP 5 LOT 47C

Abutters

N/F MARY YOUNG 656 JELLERSON ROAD EAST WAERBORO, ME 04030 BK 3168, PG 177 MAP 5 LOT 47

N/F

DOUGLAS & CONSTANCE ABBOTT 7 ABBOTT LANE EAST WATERBORO, ME 04030 MAP 5 LOT 47C

N/F

JOSEPH & JOAN WARREN C/O NORWAY SAVINGS BANK 261 MAIN ST NORWAY, ME 04268 BK 15950, PG 761 MAP 5, LOT 47D

N/F FORREST & ISABELLE ABBOTT PO BOX 21 EAST WATERBORO, ME 04030 MAP 5 LOT 48A

N/F

ROSS & JENNIFER BOOKER 6910 NW BELVIDERE PKWY KANSAS CITY, MO 64152 BK 17251 PG 322 MAP 26 LOT 6

N/F

ROBERT HAMEL JR PO BOX 294 EAST WATERBORO, ME 04030 BK 16418 PG 196 MAP 26 LOT 6A Site Plan Review Final Application Convenience Store and Office Building June 19, 2019

Attachment D Photographs

Photographs of the existing conditions of the project site are enclosed.



Photograph 1: Main Street, Looking West, from Proposed Entrance Location



Photograph 2: Main Street, Looking East, from Proposed Entrance Location



Photograph 3: Existing Townline Deli Building



Photograph 4: Existing Filling Station Canopy and Concrete Pad



Photograph 5: Rear of Existing Townline Deli Building



Photograph 6: Rear of Existing Townline Deli Building



Photograph 7: Wetland Bog at Rear of Site



Photograph 8: Existing Culvert Outlet Across Main Street



Photograph 9: Existing Gravel Access Drive to Garage



Photograph 10: Existing Garage Building on Adjacent Property

Attachment E Supporting Documents

Copies of relevant correspondence and documents pertaining to the project are enclosed.

Town, City, Plantation Street, Roa								division		Owner's Name					
	ω	are	rho	0		RT207									
SITE PLAN Scale				1" =		ft. or	as sho	wn	104	Iownhine Jeli					
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	grave Loam Sanc	× ×		lisht Bri 2.575		2.1NT	w Mineral Soil Surf	B	droch			eni sya	+ew		
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SUBSURFACE WASTEWATER DISPOSAL SYSTEM APPLICATION										Department of Health & Human Services Division of Environmental Health (207) 287-5672 Fax: (207) 287-3165			
	lown,	City, Plant	ation			Street, Ro	ad, Subo	livision		Owner's Name			
	Waterboro						202			10 workine Deli			
	SITE PLAN Scale 1"=						ft. or	as show	'n		SI	TE LOCATION	N PLAN
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	Texture	Cons	sistency	Color	M	ottling		Text	ure	Consister	ncy C	olor Mo	ttling
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Soil Cla 4 Profile	Soil Classification Slope Limiting If Ground Water 4 AII/C Bedrock Soil Classification Slope 9 Imiting Imiting												
Ki	Kundlun 23 6-7-19 Page 2 of 3 HHE-200 Rev. 02/11												






EXPLORATION COVER SHEET

The exploration logs are prepared by the geotechnical engineer from both field and laboratory data. Soil descriptions are based upon the Unified Soil Classification System (USCS) per ASTM D2487 and/or ASTM D2488 as applicable. Supplemental descriptive terms for estimated particle percentage, color, density, moisture condition, and bedrock may also be included to further describe conditions.

Drilling and Sampling Symbols:

S = Split Spoon Sample	Hyd = Hydraulic Advancement of Drilling Rods
UT = Thin Wall Shelby Tube	Push = Direct Push of Drilling Rods
SSA = Solid Stem Auger	WOH = Weight of Hammer
HSA = Hollow Stem Auger	WOR = Weight of Rod
RW = Rotary Wash	PI = Plasticity Index
SV = Lab Shear Vane (Torvane)	LL = Liquid Limit
PP = Pocket Penetrometer	MC = Natural Moisture Content
C = Rock Core Sample	USCS = Unified Soil Classification System
FV = Field Vane Shear Test	Su = Undrained Shear Strength
SP = Concrete Punch Sample	Su(r) = Remolded Shear Strength

Water Level Measurements:

Water levels indicated on the boring logs are the levels measured in the boring at the times indicated. In pervious soils, the indicated elevations are considered reliable groundwater levels. In impervious soils, the accurate determination of groundwater elevations may not be possible, even after several days of observations. Groundwater monitoring wells may be required to record accurate depths and fluctuation.

Gradation Description and Terminology:

Boulders:	Over 12 inches
Cobbles:	12 inches to 3 inches
Gravel:	3 inches to No.4 sieve
Sand:	No.4 to No. 200 sieve
Silt:	No. 200 sieve to 0.005 mm
Clay:	less than 0.005 mm

Trace: Little: Some: Silty, Sandy, etc.: Less than 5% 5% to 15% 15% to 30% Greater than 30%

Density of Granular Soils and Consistency of Cohesive Soils:

CONSISTENCY OF CC	HESIVE SOILS	DENSITY OF GRANULAR SOILS			
SPT N-value blows/ft	Consistency	SPT N-value blows/ft	Relative Density		
0 to 2	Very Soft	0 to 4	Very Loose		
2 to 4	Soft	5 to 10	Loose		
5 to 8	Firm	11 to 30	Compact		
9 to 15	Stiff	31 to 50	Dense		
16 to 30	Very Stiff	>50	Very Dense		
>30	Hard				



Project Name: Convenience Store & Post Office Location: 134 Main Street, Rockport, ME

Project Number:	19114
Date:	5/8/19

EXPLORATION SUMMARY TABLE

EXPLORATION NUMBER	SURFACE ELEVATION (ft)	BEDROCK DEPTH (ft)	BEDROCK ELEVATION (ft)	NOTES
B-1	296.0	6.5	289.0	N/A
B-2	297.0	3.0	294.0	N/A
B-3	295.0	5.0*	290.0*	*Refusal on Possible Boulder
B-4	300.0	6.0*	294.0*	*Refusal on Possible Boulder
B-5	293.0	9.6	283.0	Groundwater at 5.7'
B-6	299.0	8.0*	291.0*	*Refusal on Possible Boulder
P-1	297.0	5.1	292.0	N/A
P-2	296.0	5.2	291.0	N/A

NOTES:

1.) Explorations were performed using a truck mounted PowerProbe 9630 Pro on May 7 & 8, 2019. Borings were advanced using 2½-inch hollow stem augers with SPT split spoon sampling. Probes were advanced using 2½-inch solid stem augers.

2.) Surface elevations were estimated to the nearest foot from Site Layout Plan, Convenience Store & Post Office - dated 3/25/19 by Sitelines. Bedrock elevations are calculated based on surface elevations and measured depth to bedrock, rounded to the nearest foot.

3.) Groundwater was observed in in boring B-5, as noted in the table above. Based on the explorations, the soil consists of fill overlying ice-contact (sand & gravel) deposits with cobbles and boulders, overlying bedrock. Glacial till was encountered in

	~				SOIL BORING LOG			Boring #: B-1	
	SIIMMIT		Project: Convenience Store & Post Office		Project #:	19114			
			Location: 134 Main Street		Sheet:	1 of 1			
		GEOENGINEER	ING SERVICES	•	City, State:	City, State: East Waterboro, Maine		Chkd by:	CWC
Drilling (Drilling Co: Summit Geoengineering Services					1:	296 feet +/-		
Driller:		Craig Coolidge	e, P.E.		Reference:	Site Layout Pla	an, Convenience Store &	Post Office - Dated 3	/25/19 by Sitelines
Summit	Staff:	Erika Stewart,	, P.E.		Date started:	5/7/2019	Date Completed:	5/7/2019	
DF	RILLING	METHOD	SAI	MPLER		-	ESTIMATED GROUND V	ATER DEPTH	
Vehicle:		AMS, Truck	Length:	24" SS	Date	Depth	Elevation	Re	ference
Model:		9630 Pro	Diameter:	2"OD/1.5"ID	5/7/2019	NE	N/A	None observed	
Method:	<u>.</u>	2-¼" Augers	Hammer:	140 lb					
Hammer	Style:	N/A	Method:	ASTM D1586			_		
Depth (ft.)	No.	Pen/Rec (in)	Depth (ft)	blows/6"		DESCRIP	LE TION	Geological/ Test Data	Stratum
	S-1	24/18	0 - 2	2	Brown Gravelly	SAND to Sandy P-SM to GP-GM	GRAVEL, little Silt,		GRANULAR FILL
1				3					0.0
				2	•				
-				4					
<u>ک</u>									2'+/-
-					Auger advanced	l through occas	ional cobbles at 2' - 5'		ICE-CONTACT DEPOSIT
3_									
-									
4_									
-									
5_	6.2	10/10		0	Brown Silty SAN	D, some Grave	l, occasional cobbles		
-	5-2	18/18	5-0.5	8	and boulders, co	ompact, damp,	SM		
6				10					
-				15	Spoon refusal o	n bedrock at 6.	5'. Auger advanced 6.5'		6.5'
7_				50/0"	to 7' End of Explorati	on at 7', Auger	Refusal on Bedrock		BEDROCK 7'
-						on ac , , , age.			
8_					-				
-					-				
9									
-									
10									
-									
12									
Granul	ar Soils	Cohesive	e Soils	% Composition	NOTES:	PP = Pocket Per	netrometer, MC = Moisture	Content	Soil Moisture Condition
Blows/ft.	Density	Blows/ft.	Consistency	ASTM D2487	4	SP = Punch San	npie, $S = Split Spoon Sampl$	e	Dry: $S = 0\%$
0-4 5-10	v. LOOSE	<2 2_4	v. SOTT	< 5% Trace					$\Box u \Pi u \Pi u = 5 = 1 \text{ to } 25\%$ $Damp: S = 26 \text{ to } 50\%$
J-10 11-30	Compact	2- 1 5-8	Firm	< 5% I race 5-15% Little					Moist: $S = 51 \text{ to } 75\%$
31-50	Dense	9-15	Stiff	15-30% Some					Wet: $S = 76 \text{ to } 99\%$
>50	V. Dense	16-30	V. Stiff	> 30% With					Saturated: $S = 100\%$
		>30	Hard		Boulders = diame	ter > 12 inches, and > No 4. Sec	Cobbles = diameter < 12 in d = < No.4 and $> No.200$	ches and > 3 inches Silt/Clay = $< No 200$	
		1				unu / NU 4, Jdl	iu - < iiu ∓ aliu ≥iiu 200,	$\sin \varphi \cos \varphi = \nabla 100 200$	I

	~			SOIL BORING LOG			Boring #:	B-2	
	SILMANIT		Project:	Convenience S	Store & Post Office	Project #:	19114		
	30/1////1		Location:	Location: 134 Main Street		Sheet:	1 of 1		
		GEOENGINEER	ING SERVICE	5	City, State:	East Waterbor	o, Maine	Chkd by:	CWC
Drilling (Co:	Summit Geoer	ngineering	Services	Boring Elevation	ו:	297 feet +/-		
Driller:		Craig Coolidge	e, P.E.		Reference:	Site Layout Pla	an, Convenience Store &	Post Office - Dated 3	/25/19 by Sitelines
Summit	Staff:	Erika Stewart,	, P.E.		Date started:	5/8/2019	Date Completed:	5/8/2019	
DF	RILLING	METHOD	SA	MPLER			ESTIMATED GROUND W	ATER DEPTH	
Vehicle:		AMS, Truck	Length:	24" SS	Date	Depth	Elevation	Re	eference
Model:		9630 Pro	Diameter:	2"OD/1.5"ID	5/8/2019	NE	N/A	None observed	
Method:	a	2-¼" Augers	Hammer:	140 lb					
Hammer	Style:	N/A	Method:	ASTM D1586					
Depth (ft.)	No.	Pen/Rec (in)	Depth (ft)	blows/6"		DESCRIP	LE TION	Test Data	Stratum
_	S-1	24/15	0 - 2	3	Brown Gravelly loose, humid, S	SAND to Sandy P-SM to GP-GM	GRAVEL, little Silt,		GRANULAR FILL
1				2					
_				3	Dark brown to r and organics, fi	nottled brown S rm, damp to mo	Silty SAND, little Gravel Dist, SM		1'+/- ICE-CONTACT
2				2			Constant and an article		DEPOSIT
_	S-2	11/6	2 - 2.9	3	(wood fibers), lo	onty SAND, little cose, moist, SM	e Gravel and organics		
3				50/5"	(Spoon Refusal	on Bedrock at 2	2.9'. Auger refual at 3'.)		
					End of Exploration	ion at 3', Auger	Refusal on Bedrock		3'
_					Offset boring so	outh, auger refu	Isal at 2.7'. Offset boring		BEDROCK
4					north, auger ref	fusal at 2.5'.			
4_					*A bedrock out	crop is visible at	t edge of existing gravel		
					driveway near	horing B-2	Ledge of existing graver		
-					unveway, near	bornig b 2.			
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Granul	ar Soils	Cohesive Blows /ft	Consister	% Composition	INOTES:	PP = Pocket Per	netrometer, MC = Moisture (Lontent	Soll Moisture Condition
0_4		DIOWS/TT.	V coff	ASTM D248/	1	or = Punch San	ιιρια, ο = οριιι οροοη sample	5	U(y): S = 0% Humid: S = 1 to 25%
5-10		2-4	v. SUIL Soft	< 5% Trace					Damp: $S = 26 \text{ to } 50\%$
11-30	Compac	5-8	Firm	5-15% Little					Moist: $S = 51 \text{ to } 75\%$
31-50	Dense	9-15	Stiff	15-30% Some					Wet: $S = 76 \text{ to } 99\%$
>50	V. Dense	16-30	V. Stiff	> 30% With					Saturated: S = 100%
		>30	Hard		Boulders = diame	eter > 12 inches,	Cobbles = diameter < 12 inc	ches and > 3 inches	
					Gravel = < 3 inch	and > No 4, Sar	nd = < No 4 and >No 200, S	Silt/Clay = < No 200	

					S	OIL BORI	NG LOG	Boring #:	B-3
	SILMANIT		Project:	Convenience S	Store & Post Office	Project #:	19114		
		3011			Location:	134 Main Stree	et	Sheet:	1 of 1
		GEOENGINEER	ING SERVICES	5	City, State:	East Waterbor	o, Maine	Chkd by:	CWC
Drillina (Co:	Summit Geoer	naineerina	Services	Boring Elevation	1:	295 feet +/-		
Driller:		Craig Coolidge	e, P.E.		Reference:	Site Layout Pla	an, Convenience Store &	Post Office - Dated 3	/25/19 by Sitelines
Summit	Staff:	Erika Stewart,	, P.E.		Date started:	5/8/2019	Date Completed:	5/8/2019	
DF	RILLING I	METHOD	SAN	MPLER			ESTIMATED GROUND W	ATER DEPTH	
Vehicle:		AMS, Truck	Length:	24" SS	Date	Depth	Elevation	Re	ference
Model:		9630 Pro	Diameter:	2"OD/1.5"ID	5/8/2019	NE	N/A	None observed	
Method:		2-¼" Augers	Hammer:	140 lb					
Hammer	Style:	N/A	Method:	ASTM D1586					
Depth						SAMPI	LE	Geological/	Geological
(ft.)	No.	Pen/Rec (in)	Depth (ft)	blows/6"		DESCRIP	TION	Test Data	Stratum
	S-1	24/12	0 - 2	1	Brown Gravelly	SAND, little to t	trace Silt, loose,		GRANI II AR ETI I
-				2		551			
1_				_	Dark brown to n and Gravel, trac	nottled brown S e rootlets/orga	SILT-SAND, little Clay nics, loose, moist, SM-ML		0.8' ICE-CONTACT
-				1					DEPOSIT
2				2					
3									
<u> </u>					-				
-									
4_					Auger refusal or	n houlder at 4'			
_					Offset boring, a	uger refusal at	4'.		
5					Offset boring, a refusal at 5.	Offset boring, auger advanced through 2 cobbles, refusal at 5.			
-					End of Explorati	on at 5', Auger	Refusal on Boulder or		5' BOULDER OR
-					Deurock.				BEDROCK
6_					-				
-									
7									
8									
<u> </u>					-				
-									
9_					-				
-									
10									
-					1				
12					4				
Granul	ar Soils	Cohesive	e Soils	% Composition	NOTES:	PP = Pocket Per	netrometer, MC = Moisture (Content	Soil Moisture Condition
Blows/ft.	Density	Blows/ft.	Consistency	ASTM D2487		SP = Punch San	nple, S = Split Spoon Sample	2	Dry: S = 0%
0-4	V. Loose	<2	V. soft		1				Humid: S = 1 to 25%
5-10	Loose	2-4	Soft	< 5% Trace					Damp: S = 26 to 50%
11-30	Compact	5-8	Firm	5-15% Little					Moist: S = 51 to 75%
31-50	Dense	9-15	Stiff	15-30% Some					Wet: S = 76 to 99%
>50	V. Dense	16-30	V. Stiff	> 30% With					Saturated: S = 100%
		>30	Hard		Boulders = diame	ter > 12 inches,	Cobbles = diameter < 12 inc	ches and > 3 inches	
					Gravel = < 3 inch	and > No 4, Sar	$d = \langle No 4 and \rangle No 200, S$	Silt/Clay = < No 200	

				S	OIL BORI	NG LOG	Boring #:	B-4	
		SIM	MIT		Project: Convenience Store & Post Office			Project #:	19114
	GEOENGINEERING SERVICES		Location:	Location: 134 Main Street Shee		Sheet:	1 of 1		
		GEOENGINEER	ING SERVICES	•	City, State:	East Waterbor	o, Maine	Chkd by:	CWC
Drilling (Co:	Summit Geoer	ngineering	Services	Boring Elevation	1:	300 feet +/-		
Driller:		Craig Coolidge	e, P.E.		Reference:	Site Layout Pla	an, Convenience Store & I	Post Office - Dated 3	/25/19 by Sitelines
Summit	Staff:	Erika Stewart,	, P.E.		Date started:	5/8/2019	Date Completed:	5/8/2019	
DF	RILLING	METHOD	SAI	MPLER		-	ESTIMATED GROUND W	ATER DEPTH	
Vehicle:		AMS, Truck	Length:	24" SS	Date	Depth	Elevation	Re	eference
Model:		9630 Pro	Diameter:	2"OD/1.5"ID	5/8/2019	NE	N/A	None observed	
Method:		2-¼" Augers	Hammer:	140 lb					
Hammer	Style:	N/A	Method:	ASTM D1586					
Depth (ft.)	No.	Pen/Rec (in)	Depth (ft)	blows/6"	-	SAMPI DESCRIP	LE TION	Geological/ Test Data	Geological Stratum
	SP-1	12/12	0 - 1	PUSH	Bituminous Pave	ement = 4"			PAVEMENT
1					Brown SAND, so damp, SP-SM	ome Gravel, tra	ce Silt, compact, humid to	Gravel = 24% Sand = 71%	0.3' GRANULAR FILL
-	S-1	12/6	1' - 2'	4	Same as above,	compact, dam	p, SP-SM	MC = 3.1%	
2				15					
-				50/0"	Spoon refusal of Frequent cobble	n cobble at 2'. es from 2' to 6'.			2'+/- ICE-CONTACT DEPOSIT
3_									(Portions Reworked)
4									
-					-				
5_							little Cilt. comment		
-	S-2	12/8	5' - 6'	9	damp to moist,	SAND, some to SM	nute Sitt, compact,		
6_				50/0"	Spoon refusal at	t 6'. Auger adva	anced from 6' - 6.3'		6'
-					End of Explorati	on at 6.3', Aug	er Refusal on Boulder		BOULDER OR BEDROCK 6.3'
7_					or Bedrock				
8									
_					-				
9_					-				
-					-				
10_									
12]				
Granul	ar Soils	Cohesive	e Soils	% Composition	NOTES:	PP = Pocket Per	netrometer, MC = Moisture C	Content	Soil Moisture Condition
Blows/ft.	Density	Blows/ft.	Consistency	ASTM D2487	4	SP = Punch San	nple, S = Split Spoon Sample	2	Dry: S = 0%
0-4	V. LOOSE	<2	V. soft	< E0/ T					Humid: $S = 1$ to 25%
5-10	LOOSE	2-4 E 0	Soft	< 5% I race					Damp: $S = 26 \text{ to } 50\%$
31-50	Compact	5-8 0,15	rirm s⊧iff	5-15% Little					Mot: $S = 51 \text{ to } /5\%$
>50	V. Dense	16-30	V Stiff	> 30% With					Saturated: S = 100%
2.50		>30	Hard	> 5070 WILL	Boulders = diame	ter > 12 inches,	Cobbles = diameter < 12 inc	thes and > 3 inches	Suturateur. 5 - 100 /0
		1			Graver = < 3 inch	anu > 110 4, Sar	u = < 100 + and > 100 200, S		

					S	OIL BORI	NG LOG	Boring #:	B-5
	SUMMIT		Project:	Convenience S	Store & Post Office	Project #:	19114		
			Location:	Location: 134 Main Street		Sheet:	1 of 1		
		GEOENGINEER	ING SERVICES	5	City, State:	East Waterbor	o, Maine	Chkd by:	CWC
Drilling (Co:	Summit Geoe	naineerina	Services	Boring Elevation	ו:	293 feet +/-		
Driller:		Craig Coolidge	e, P.E.		Reference:	Site Layout Pla	an, Convenience Store & F	Post Office - Dated 3	/25/19 by Sitelines
Summit	Staff:	Erika Stewart,	, P.E.		Date started:	5/8/2019	Date Completed:	5/8/2019	<u> </u>
DF	RILLING	METHOD	SA	MPLER			ESTIMATED GROUND W	ATER DEPTH	
Vehicle:		AMS, Truck	Length:	24" SS	Date	Depth	Elevation	Re	eference
Model:		9630 Pro	Diameter:	2"OD/1.5"ID	5/8/2019	5.7'	285'+/-	Measured in augers	5
Method:		2-1/4" Augers	Hammer:	140 lb					
Hammer	Style:	N/A	Method:	ASTM D1586					
Depth						SAMP	LE	Geological/	Geological
(ft.)	No.	Pen/Rec (in)	Depth (ft)	blows/6"		DESCRIP	TION	Test Data	Stratum
_	S-1	24/15	0' - 2'	6	Brown Gravelly Silt, compact, co	SAND to Sandy obbles, humid,	GRAVEL, little to trace SP-SM to GP-GM		GRANULAR FILL
1_				8					
_				8	compact, damp,	y SAND, some t , SM	o little Gravel, cobbles,		I'+/- ICE-CONTACT
2_				11	-				DEPOSIT
_					-				
3_					-				
_					-				
4_					-				
_					-				
5_					Light brown SA	ND some Grave	el trace Silt compact		
-	S-2	4/4	5' - 5.3'	50/4"	to dense, damp	to moist, SP	e/boulder. Augered		
6					through cobbles	s at 5.3' - 6'.)	.,		6'+/-
-					Gray SILT-SANE	D, some Gravel,	cobbles, trace Clay, SM-MI		GLACIAL TILL
7_					-	,			
-					-				
8_					-				
-									
9					-				
_					End of Explorati	ion at 9.6', Aug	er Refusal on Bedrock		9.6'
10					-				BEDROCK
-									
12_					-				
Granul	ar Soils	Cohesive	e Soils	% Compositior	NOTES:	PP = Pocket Per	netrometer, MC = Moisture C	Content	Soil Moisture Condition
Blows/ft.	Density	Blows/ft.	Consistency	ASTM D2487]	SP = Punch San	nple, S = Split Spoon Sample	2	Dry: S = 0%
0-4	V. Loose	<2	V. soft]				Humid: S = 1 to 25%
5-10	Loose	2-4	Soft	< 5% Trace					Damp: S = 26 to 50%
11-30	Compact	5-8	Firm	5-15% Little					Moist: S = 51 to 75%
31-50	Dense	9-15	Stiff	15-30% Some					Wet: S = 76 to 99%
>50	V. Dense	16-30	V. Stiff	> 30% With	L				Saturated: S = 100%
		>30	Hard		Boulders = diame Gravel = < 3 inch	eter > 12 inches, and > No 4, Sar	Cobbles = diameter < 12 inc ad = < No 4 and >No 200, S	hes and > 3 inches ilt/Clay = < No 200	

	~				S	SOIL BORING LOG			B-6	
		SIM	MIT		Project: Convenience Store & Post Office			Project #:	19114	
		GEOENGINEER	ING SERVICE	20	Location:	Location: 134 Main Street		Sheet:	Sheet: 1 of 1	
		GEOENGINEER	ING SERVICES		City, State:	East Waterbor	o, Maine	Chkd by:	CWC	
Drilling (Co:	Summit Geoe	ngineering	Services	Boring Elevatior	ו:	299 feet +/-			
Driller:		Craig Coolidge	e, P.E.		Reference:	Site Layout Pla	an, Convenience Store &	Post Office - Dated	3/25/19 by Sitelines	
Summit	Staff:	Erika Stewart,	, P.E.		Date started:	5/8/2019	Date Completed:	5/8/2019		
DF	RILLING	METHOD	SA	MPLER			ESTIMATED GROUND W	ATER DEPTH		
Vehicle:		AMS, Truck	Length:	24" SS	Date	Depth	Elevation	R	eference	
Model:		9630 Pro	Diameter:	2"OD/1.5"ID	5/8/2019	NE	N/A	None observed		
Method:		2-1/4" Augers	Hammer:	140 lb						
Hammer	Style:	N/A	Method:	ASTM D1586						
Depth						SAMPI	LE	Geological/	Geological	
(ft.)	No.	Pen/Rec (in)	Depth (ft)	blows/6"		DESCRIP	TION	Test Data	Stratum	
	S-1	24/12	0' - 2'	1	Dark brown Silty	y SAND, rootlet	s, soft, damp, SM		TOPSOIL	
1				3	Brown Gravelly	SAND, little Silt	, loose, humid to damp,		0.3' GRANIJI AR FILL	
				5						
-				4	-					
2_					-					
-					-					
3_									3'+/-	
_									ICE-CONTACT	
4									DEPOSIT (Portions Reworked)	
-					-					
-	S-2	3/3	4.5' - 4.7'	50/3"	Spoon refusal o	n cobble/bould	er at 4.7'. Rock fragment	s		
5_		-,-		,-	in spoon tip. Auger refusal at	t 5' on boulder.	Offset 2' east, auger			
_					refusal at 5.5'. (Offset 2' west, o	cobble at 6.9', auger			
6							OCK.			
7										
<i>'</i> -										
-					-					
8_					End of Explorati	ion at 8.0' Aug	er Refusal on Boulder		8.0'	
_					or Bedrock				BOULDER OR	
9									BEDROCK	
10										
10_					-					
-					-					
12					4					
Granul	ar Soils	Cohesive	e Soils	% Compositior	NOTES:	PP = Pocket Per	netrometer, MC = Moisture	Content	Soil Moisture Condition	
Blows/ft.	Density	Blows/ft.	Consistency	ASTM D2487		SP = Punch San	nple, S = Split Spoon Sampl	е	Dry: S = 0%	
0-4	V. Loose	<2	V. soft		1				Humid: S = 1 to 25%	
5-10	Loose	2-4	Soft	< 5% Trace					Damp: S = 26 to 50%	
11-30	Compact	5-8	Firm	5-15% Little					Moist: S = 51 to 75%	
31-50	Dense	9-15	Stiff	15-30% Some	2				Wet: S = 76 to 99%	
>50	V. Dense	16-30	V. Stiff	> 30% With					Saturated: S = 100%	
		>30	Hard		Boulders = diame	eter > 12 inches,	Cobbles = diameter < 12 in	ches and > 3 inches		
					Gravel = < 3 inch	and > No 4, Sar	$d = \langle No \ 4 \ and \rangle No \ 200, 3$	Silt/Clay = < No 200		

STATE OF GEORGIA

Secretary of State Corporations Division **313 West Tower** 2 Martin Luther King, Jr. Dr. Atlanta, Georgia 30334-1530

ANNUAL REGISTRATION

Electronically Filed Secretary of State Filing Date: 3/28/2019 1:49:46 PM

BUSINESS INFORMATION								
CONTROL NUMBER 08010302								
BUSINESS NAME NEW HORIZON MANAGEMENT COMPANY, LLC								
BUSINESS TYPE	Domestic Limited Liability Company							
EFFECTIVE DATE	03/28/2019							
El C								
PRINCIPAL OFFICE ADDRES	S							
ADDRESS	3391 White Sulphur Road, Gainesville, USA							
REGISTERED AGENT								
NAME	ADDRESS COUNTY							
Peck III, Daniel J.	3391 White Sulphur Road, Gainesville, GA, 30501, USA Hall							
AUTHORIZER INFORMATIO	N							
AUTHORIZER SIGNATURE	Karen D Peck							
AUTHORIZER TITLE	Organizer Organizer							
	1776							





	Y			
	MAGNETIC			
		1. 06-19-2019 SUBMITTED 1	to town for final review	MUL
		PROJECT: CONVENIENCE	K TURNING PLAN	DING
		134 MAIN STREE PREPARED FOR: NEW HORIZONS 3391 WHITE SULPH	T, EAST WATERBORO, ME MANAGEMENT COMPANY, UR ROAD, GAINESVILLE, (04030 LLC GA 30501
60	JOSEPH		SITELIN 119 PURINTON ROAD, BRUINSWICK, MAINE 207.725.1200 PLANNERS • LAND SURV	JES SUITE A 04011 EYORS
	MARDEEN 12828 CONST ONAL 06-19-19	FIELD WK: OTHERS DRN BY: JJM CHD BY: CYN DATE: 03-25-19	SCALE: 1"=30' SF JOB #: 3841 MAP/LOT: 5/37C&48-1 FILE: 3841-SITE	^{неет:}

Attachment F Supporting Graphics

This attachment includes supporting materials and graphics for the application. This includes an excerpt of the FEMA flood rate insurance map (FIRM) and reduced size copies of the zoning map and tax maps. An excerpt of the applicable USGS 7.5 minute quadrangle map is provided for reference.





Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
СоВ	Colton gravelly loamy coarse sand, 0 to 8 percent slopes	0.1	3.5%
CrB	Croghan loamy sand, 0 to 8 percent slopes	0.5	15.7%
HnE	Hermon sandy loam, 15 to 60 percent slopes, extremely stony	2.5	80.8%
Totals for Area of Interest	·	3.1	100.0%

York County, Maine

CoB—Colton gravelly loamy coarse sand, 0 to 8 percent slopes

Map Unit Setting

National map unit symbol: 9k58 Elevation: 10 to 2,000 feet Mean annual precipitation: 30 to 48 inches Mean annual air temperature: 37 to 46 degrees F Frost-free period: 90 to 160 days Farmland classification: Farmland of statewide importance

Map Unit Composition

Colton and similar soils: 85 percent Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Colton

Setting

Landform: Outwash terraces Landform position (two-dimensional): Summit Landform position (three-dimensional): Tread Down-slope shape: Linear Across-slope shape: Linear Parent material: Sandy-skeletal glaciofluvial deposits derived from granite and gneiss

Typical profile

Oa - 0 to 2 inches: highly decomposed plant material

H1 - 2 to 12 inches: gravelly loamy sand

H2 - 12 to 20 inches: gravelly coarse sand

H3 - 20 to 65 inches: very gravelly coarse sand

Properties and qualities

Slope: 0 to 8 percent Depth to restrictive feature: More than 80 inches Natural drainage class: Excessively drained Capacity of the most limiting layer to transmit water (Ksat): Moderately high to very high (1.42 to 14.17 in/hr) Depth to water table: More than 80 inches Frequency of flooding: None Frequency of ponding: None Available water storage in profile: Very low (about 2.3 inches)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 3s Hydrologic Soil Group: A

USDA

Hydric soil rating: No

Data Source Information

Soil Survey Area: York County, Maine Survey Area Data: Version 17, Sep 11, 2018



<u>Attachment G</u> Stormwater Management

A copy of the stormwater analysis is included in accordance with the Waterboro Zoning Ordinance.

Stormwater Management Plan Convenience Store and Office Building 134 Main Street, East Waterboro, Maine





New Horizons Management Company, LLC (herein referred to as Applicant) is proposing construction of two (2) buildings; a 6,200 s.f. convenience store, and a 1,825 s.f. office building, along with associated access drive and parking areas. The proposed development will result in approximately 55,191 sq. ft. (1.27 acres) of impervious area, or an increase of 19,275 s.f. (0.44 acres). A portion of the stormwater runoff from the site will be conveyed to a grassed underdrained soil filter for treatment and detention prior to discharge to the adjacent wetland complex.

As the project results in less than one (1) acre of new impervious area since 2005, but more than an acre of disturbance, a Stormwater Law Permit-by-Rule (PBR) will be required from the Maine Department of Environmental Protection (MDEP). Per strict adherence to the Town of Waterboro Zoning Ordinances, stormwater runoff from 95% of the impervious area and 80% of the developed area is required to be treated. Per discussions with the Town Planner, he agrees that as the project is the redevelopment of an existing development, treatment of only the new impervious area is needed to meet the intent of the Ordinance. A waiver from that portion of the Ordinance is being requested as part of the Site Plan Review application to the Town.

The proposed stormwater management plan has been designed to mitigate any stormwater runoff impacts from the project and direct the stormwater runoff to existing drainage water courses and avoid impacts to abutting properties.

Study Methodology

Topographical data was obtained from on-the-ground survey collected by Stephen Everett and supplemented by survey from Sitelines, PA. Hydrologic boundaries were generated using the topographic mapping and the drainage patterns were verified by a site reconnaissance visit.

Surficial soils located in the vicinity of the site were obtained from the United States Department of Agriculture Natural Resources Conservation Service Soil Survey Geographic (SSURGO) Database. The Applicant's parcel includes the soil classifications listed below. Soils units found in the development area are primarily Lamoine.

	SOILS TYPES IN LOCAL STUDY AREA					
Soils Series	Symbol(s)	Hydrologic Group (HSG) **				
Colton	СоВ	Α				
Croghan	CrB	Α				
Hermon	HnE	Α				

**Hydrologic Soils Group taken from SCS TR-55 Manual

Water Quantity

Per the Town of Waterboro Zoning Ordinance, a comparison of pre- and post-development peak stormwater runoff rates is needed to determine what impact, if any, there is to downstream drainageways and abutting properties, as a result of the project.

Flooding

The project area is located in Zone C (Areas of minimal flooding) of the Flood Insurance Rate Maps (FIRMs) for York County, Maine. The project area is located on Panel 20 of 20 (Community Panel 230199-0020-C, Effective February 1, 1985). An excerpt of the applicable FIRM is included as an attachment to this section. There is no impact from flooding anticipated for this project.

Off-Site Watersheds

The watershed area analyzed for the pre- and post-development analysis consists of the project site, a portion of the abutting property to the south, and a portion of Main Street directly adjacent to the site. There is a 12-inch culvert which discharges across Main Street (Rt. 202) and discharges on the property. Without any outlet, the stormwater ponds up and infiltrates into the existing soils. Looking at aerial topography, I anticipate that the natural drainage used to flow from the northwest corner to the southwest corner of the property, however development of the property has resulted in the culvert being hydraulically "cut-off" from the large wetland complex in the southeast corner. As part of the drainage design, the culvert which crosses Main Street (Rt. 202) will be incorporated into the new drainage system and directed to the large wetland complex. The stormwater from this culvert was not analyzed or incorporated into our pre- or post-development drainage calculations. It is anticipated that incorporating the culvert into the drainage system will alleviate existing stormwater impacts on adjacent properties.

Stormwater Analysis Subcatchments

Pre-Development Conditions

A summary of the subcatchments is provided below:

- Subcatchment 1 represents approximately 1.37 acres comprised of the Townline Deli building and parking areas, a portion of the access drive for the garage on the abutting property, lawn area, and wooded areas. Stormwater runoff is conveyed overland towards the large wetland complex in the southwest corner of the property (POI#1).
- Subcatchment 2 represents approximately 1.19 acres comprised of a portion of the access drive for the garage on the abutting property, lawn area, and wooded area. Stormwater runoff is conveyed overland to low areas on the eastern portion of the property. This area does not have any outlet and the stormwater infiltrates into the soils.

Post-Development Conditions

Under post-development conditions, two (2) buildings will be constructed; a 6,200 s.f. convenience store, and a 1,825 s.f. office building, along with associated access drive and parking areas, resulting in approximately 55,191 sq. ft. (1.27 acres) of impervious area, or an increase of 19,275 s.f. (0.44 acres). Stormwater runoff from a portion of the impervious area will be directed to a grassed underdrained soil filter for treatment and attenuation of the anticipated flows. A summary of the subcatchments is provided below:

- Subcatchment 10 represents approximately 0.97 acres comprised the new buildings, paved areas, and lawn areas. Stormwater runoff is conveyed via catch basins to the grassed underdrained soil filter. From the soil filter, the stormwater runoff will be directed to the large wetland complex in the southwest corner of the property (POI#1).
- Subcatchment 11 represents approximately 0.88 acres comprised of paved areas and lawn areas. Stormwater runoff is conveyed via catch basins towards the large wetland complex in the southwest corner of the property (POI#1).

Subcatchment 12 represents approximately 0.75 acres comprised of Main Street and lawn areas. Stormwater runoff is conveyed overland to low areas on the eastern portion of the

property. This area does not have any outlet and the stormwater infiltrates into the soils.

Results

A comparison of pre- and post-development peak stormwater runoff rates at the Points of Interest is presented in the following table. Peak runoff rates were estimated for the 2, 10, and 25-year, 24-hour storm events. Point of Interest 1 is located at the large wetland complex.

Design	Pre	Post	Change	
Storm				
2-Year	2.37	1.62	-0.75	
10-Year	3.43	2.81	-0.62	
25-Year	4.26	5.45	+1.19	

As shown in the table, the peak runoff rate is decreased for the 2- and 10-year storm events and increased for the 25-year storm event at the Point of Interest. Per the Waterboro Zoning Ordinance, the *"Stormwater management system must include treatment measures that will mitigate for the increased frequency and duration of channel erosive flows due to runoff from smaller storms..."* With the reduction in peak runoff rates during the smaller storms, the proposed design meets the design standards as outlined within the Ordinance. Furthermore, due to the size of the wetland complex, the increase in peak runoff rate further downstream.

Water Quality

The proposed project will result in approximately 55,191 sq. ft. (1.27 acres) of impervious area, or an increase of 19,275 s.f. (0.44 acres). Runoff from a portion of the impervious areas will be conveyed to a grassed underdrained soil filter for water quality treatment. Approximately 35,160 s.f. (0.81 acres), or 182%, of the new impervious areas will be conveyed this grassed underdrained soil filter. The remaining 20,031 s.f. (0.46 acres) of impervious area will remain untreated and will either be directed to the large wetland complex in the southwest corner or the low spot along the eastern portion of the property.

Approximately 29,550 s.f. (0.68 acres) of new developed area will be created as a result of the proposed development. Approximately 41,539 s.f. (0.95 acres), or 141% of the total developed area, will be conveyed to the grassed underdrained soil filter. The remaining developed area will remain untreated and will either be directed to the large wetland complex in the southwest corner or the low spot along the eastern portion of the property.

Grassed Underdrained Soil Filter

A Grassed Underdrained Soil Filter will be utilized for water quality treatment. The soil filters have been sized to capture and detain a water quality capture volume of 1" of runoff from impervious area and 0.4" of runoff from landscaped areas and allows it to filter through the filter material to an underdrain system. A filtration rate of 2.4 inches/hour was used in modeling the area as a conservative estimate for ponding evaluation. This rate considers a decrease in filtration rate that will likely be realized over time. An overflow structure is proposed that will safely bypass runoff volumes in excess of the 1-inch storm.

The impervious and landscaped areas tributary to the grassed underdrained soil filter and the required sizing are summarized in the table below.

Impervious Area and Volume Requirements

	(a)	(b)	(c)	(d)	(e)	(i)
Sub- Area	Impervious area (sq. ft.)	Required Storage (cu. ft.)	Landscaped Area (sq. ft.)	Required Storage (cu. ft.)	Total Storage Required / Provided (cu. ft.)	Filter Area Required / Provided (sq. ft.)
	(from plan)	(a)x0.083'	(from plan)	(c)x0.033'	(b)+(d)	(a)x0.05+ (c)x0.02+
GUSF*	35,160	2,930	7,254	242	3,172 / 4,023	1,903 / 2,166

*Grassed Underdrained Soil Filter

Conclusion

Based on the stormwater analysis, development of the subject property results in a decrease from the predevelopment peak runoff rates during the 2- and 10-year storm events and meets the design standards as outlined within the Waterboro Zoning Ordinance.

Runoff from 182% of the new impervious area and 141% of the total developed area will be captured and conveyed to forested buffers for water quality treatment. By capturing and treating runoff from the impervious surfaces and developed areas the project likewise meets the requirements of the Waterboro Zoning Ordinance.

Attachment A – FEMA Flood Map

Attachment B – Stormwater Facilities Inspection and Maintenance Plan

 $\label{eq:constraint} Attachment \ C-Pre-\ and \ Post-Development \ HydroCAD \ Report$

Attachment D- Pre- and Post-Development Watershed Plans



Convenience Store and Office Building 134 Main Street, East Waterboro, Maine

STORMWATER FACILITIES INSPECTION AND MAINTENANCE PLAN

1.0 <u>General</u>

This stormwater facilities maintenance plan has been prepared in support of the Site Plan Review application for the construction of a convenience store and office building to be located at 134 Main Street in East Waterboro, Maine. The requirements of this plan shall be incorporated into the efforts associated with the development including construction and ongoing operations.

2.0 <u>Best Management practices</u>

2.1 Best Management Practices

During Construction, a stabilized construction entrance, sediment barrier, erosion control blanket and/or erosion control mix, seeding, and mulching practices will be used in accordance with the Maine Department of Environmental Protection Best Management Practices (BMP) manual during construction and until a stabilized condition exists.

After Construction, stormwater BMPs will include housekeeping and physical measures described herein, including a grassed underdrained soil filter, sweeping of paved surfaces, maintenance of riprap erosion control and maintenance of storm drain pipes and outfalls.

The stormwater maintenance management for this project will be performed consistent with the two references listed below and as amended in this manual. Where standards are not consistent, the more stringent requirement shall apply.

2.2 References

The primary references for the stormwater management design were as follows:

- 1 "Stormwater Management for Maine", Maine Department of Environmental Protection No. DEPLW0738, January 2006.
- 2 "Maine Erosion and Sedimentation Best Management Practices", Maine Department of Environmental Protection, current edition on-line.

3.0 MAINTENANCE OF STORMWATER FEATURES

3.1 General Responsibilities

The Contractor will be responsible for maintaining the stormwater BMPs and facilities until the construction phase of the project is complete and the site is permanently stabilized and accepted by the Applicant. These efforts shall include maintenance of temporary and permanent stormwater features and addressing interim site conditions as necessary. After acceptance of the development, the Applicant will be responsible for maintaining the permanent stormwater features as shown on the plan.

Stormwater Facilities Inspection and Maintenance Plan Convenience Store and Office Building 134 Main Street, East Waterboro, Maine Page 2 of 3

The Point of Contact for the Applicant is as follows:

David Pendleton 134 Main Street East Waterboro, Maine Phone: 207-247-1031

3.1 General Requirements

The general requirements for this stormwater maintenance management manual will meet the standards of Reference No.1, specific to the water quality feature concerned. Additional maintenance requirements are identified in the following narratives.

3.2 Specific Maintenance Requirements

The following specific maintenance requirements apply to stormwater features as follows:

3.2.1 Grassed Underdrained Soil Filter Basin

The maintenance of grassed underdrained soil filter basins shall be in accordance with Section 7.1.6 of Reference No. 1.

- Soil Filter Inspection: The soil filter shall be inspected after every major storm in the first few months to ensure proper function. Thereafter, the filter should be inspected at least once every month to ensure that it is draining completely between 24 and 72 hours after a storm event.
- Inspect the filter basin's interior side slopes twice annually. Immediately repair any sign of erosion or bare areas to assure a vigorous growth of vegetation for the stability of the slope and proper function.
- Inspect the basin's overflow outlet twice annually and after every major rainfall event, and remove any collected trash or debris on or in the outlet.
- Sediment Removal: The grass buffer area vegetation should be inspected at least once per year, preferably in the spring. Debris and sediment build-up should be removed from the buffer when noticeable accumulation has occurred.
- Restoring Infiltrative Capacity: The surface of the soil filter may clog with fine sediments over time. Maintenance of good grass cover should minimize this; however, if runoff ponded in the basin does not drain within 48 hours, rototilling of the top of the soil bed may be required to re-establish the soil's filtration capacity.
- The top several inches of the filter shall be replaced with fresh material when water ponds on the surface of the bed for more than 72 hours. The removed sediments should be disposed in an acceptable manner.
- Mowing of the filter vegetated areas should be performed no more than two times per growing season to maintain grass heights no less than 6 inches.
- Fertilization of the underdrained filter area should be avoided.
- Harvesting and pruning of excessive growth should be done occasionally to control unwanted or invasive plants.

3.2.2 Storm Drain System

• Piped drainage systems shall be inspected in spring and late fall, and after heavy rains to remove any obstructions to flow; remove accumulated sediments and debris at the inlet, at the

Stormwater Facilities Inspection and Maintenance Plan Convenience Store and Office Building 134 Main Street, East Waterboro, Maine Page 3 of 3

outlet, and within the conduit; and to repair any erosion damage at the culvert's inlet and outlet. Sediment should be removed when its level exceeds 20% of the pipe diameter. Hydraulic flushing or any mechanical means may accomplish sediment removal. Care shall be taken to contain the sediment at the pipe outlet.

3.2.3 Drainage Structures

- The maintenance of structures shall be performed monthly to ensure proper function.
- Debris and trash shall be removed from the structure when present.
- Sediment build-up in the sump should be removed when accumulation within 1 foot of the outlet pipe is observed.

3.2.4 Vegetative Surfaces

- For most vegetative surfaces, grass should be mowed on a regular basis so that grass height does not exceed 6 inches. Any erosion rills, gullies, or bare spots should be seeded or sodded to re-establish the turf cover.
- Buffer, screening, and decorative landscaping should be inspected for health on a regular basis. Pruning, weeding, feeding, and mulching shall be performed on a regular basis.

3.2.5 Paved Surfaces

• Accumulations of winter sand along impervious areas shall be cleared at least once a year, preferably in the spring. Accumulations on pavement may be removed by pavement sweeping. Accumulations of sand along the edge of paved areas may be removed by grading excess sand to the pavement edge and removing it manually or by a front-end loader.

4.0 INSPECTION AND MAINTENANCE CHECKLIST

4.1 Maintenance Frequency

Notwithstanding any other schedule noted herein, general inspections should be conducted monthly during wet weather conditions from March to November.

4.2 Inspection and Maintenance Checklist

Inspection of the stormwater facilities shall be completed by individual qualified by experience or training to assess their condition and performance. Maintenance actions required will be documented, completed and inspected by individuals trained or experienced in such maintenance. An inspection and maintenance checklist specific to the facilities for this development is included.

Convenience Store and Office Building 134 Main Street, East Waterboro, Maine Stormwater Inspection and Maintenance Log

Performed by:__

Date:__

Feature	Description of maintenance	Recorded Observation
Grassed Underdrained Soil Filter Basin	Inspect for evidence of excessive sediment deposits.	
	Inspect side slopes for signs of erosion or base areas.	
	Inspect overflow for presence of sediment and/or trash.	
Storm Drain & Detention Pipes	Inspect for evidence of sediment	
	Inspect for clogging debris and material	
Drainage Structures	Inspect for presence of sediment in traps; remove sediment if within 1 foot of outlet invert.	
	Inspect frame and grate to verify grate is flush with finish grade.	
	Remove trash and debris	
Vegetative Surfaces	Inspect for vegetative cover of at least 85%	
Paved Surfaces	Remove winter sand	



Summary for Subcatchment 1S:

Runoff = 2.37 cfs @ 12.02 hrs, Volume= 0.199 af, Depth= 1.75"

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-48.00 hrs, dt= 0.10 hrs Type III 24-hr 2-year Rainfall=3.20"

A	Area (sf)	CN	Description						
	35,135	98	Paved park	ing, HSG A	N				
	16,538	39	>75% Gras	s cover, Go	bod, HSG A				
	8,022	30	Woods, Go	oods, Good, HSG A					
	59,695		Weighted A	verage					
24,560 41.14% Pervious Area				vious Area					
	35,135		58.86% Imp	pervious Are	ea				
Tc	Length	Slope	e Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)) (ft/sec)	(cfs)					
5.0					Direct Entry,				
					-				

Summary for Subcatchment 2S:

Runoff = 0.86 cfs @ 12.02 hrs, Volume= 0.072 af, Depth= 0.73"

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-48.00 hrs, dt= 0.10 hrs Type III 24-hr 2-year Rainfall=3.20"

Area (sf) C	N D	escription		
39,2	54 3	30 B	rush, Goo	d, HSG A	
12,7	40 9	98 P	aved parki	ing, HSG A	
51,9	94	V	/eighted A	verage	
39,2	54	7	5.50% Per	vious Area	
12,7	40	24	4.50% Imp	ervious Are	ea
Tc Len (min) (fe	igth S eet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,

Summary for Reach 1R: POI#1

Inflow Area	a =	1.370 ac, 5	8.86% Imp	ervious,	Inflow Dep	pth = 1.	75" fo	r 2-y	ear event
Inflow	=	2.37 cfs @	12.02 hrs,	Volume	=	0.199 af		-	
Outflow	=	2.37 cfs @	12.02 hrs,	Volume	=	0.199 af,	Atten=	:0%,	Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.10 hrs

Summary for Subcatchment 1S:

Runoff = 3.43 cfs @ 12.02 hrs, Volume= 0.297 af, Depth= 2.60"

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-48.00 hrs, dt= 0.10 hrs Type III 24-hr 10-year Rainfall=4.60"

A	rea (sf)	CN	Description						
	35,135	98	Paved park	ing, HSG A	N				
	16,538	39	>75% Gras	s cover, Go	bod, HSG A				
	8,022	30	Woods, Go	oods, Good, HSG A					
	59,695		Weighted A	verage					
	24,560 41.14% Pervious Area								
	35,135		58.86% Imp	pervious Are	ea				
Tc	Length	Slope	e Velocity	Capacity	Description				
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)					
5.0					Direct Entry,				
					-				

Summary for Subcatchment 2S:

Runoff = 1.24 cfs @ 12.02 hrs, Volume= 0.106 af, Depth= 1.07"

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-48.00 hrs, dt= 0.10 hrs Type III 24-hr 10-year Rainfall=4.60"

Area (sf)	CN	Description		
39,254	30	Brush, Goo	d, HSG A	
12,740	98	Paved park	ing, HSG A	A
51,994		Weighted A	verage	
39,254		75.50% Per	rvious Area	1
12,740		24.50% Imp	pervious Are	ea
Tc Length	Slop	e Velocity	Capacity	Description
(min) (feet)	(ft/f	t) (ft/sec)	(cfs)	
5.0				Direct Entry,

Summary for Reach 1R: POI#1

Inflow Area	a =	1.370 ac, 5	58.86% Imp	ervious,	Inflow De	pth = 2	2.60"	for 10-	year event
Inflow	=	3.43 cfs @	12.02 hrs,	Volume	=	0.297 a	f		-
Outflow	=	3.43 cfs @	12.02 hrs,	Volume	=	0.297 a	f, At	ten= 0%,	Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.10 hrs

Summary for Subcatchment 1S:

Runoff = 4.26 cfs @ 12.02 hrs, Volume= 0.379 af, Depth= 3.32"

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-48.00 hrs, dt= 0.10 hrs Type III 24-hr 25-year Rainfall=5.70"

A	Area (sf)	CN	Description						
	35,135	98	Paved parking, HSG A						
	16,538	39	>75% Grass cover, Good, HSG A						
	8,022	30	Woods, Go	Woods, Good, HSG A					
	59,695		Weighted Average						
	24,560	41.14% Pervious Area							
35,135 58.86% Impervious Area				pervious Are	ea				
Tc	Length	Slope	e Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)) (ft/sec)	(cfs)					
5.0					Direct Entry,				
					-				

Summary for Subcatchment 2S:

Runoff = 1.54 cfs @ 12.02 hrs, Volume= 0.136 af, Depth= 1.37"

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-48.00 hrs, dt= 0.10 hrs Type III 24-hr 25-year Rainfall=5.70"

Area (s	f) CN	N D	escription				
39,25	64 30) В	rush, Goo	d, HSG A			
12,74	.0 98	3 P	aved park	ing, HSG A			
51,99)4	Weighted Average					
39,25	54	75.50% Pervious Area					
12,74	-0	24	4.50% Imp	pervious Are	ea		
				-			
Tc Leng	gth S	lope	Velocity	Capacity	Description		
(min) (fee	et) ((ft/ft)	(ft/sec)	(cfs)			
5.0					Direct Entry,		
			_	-			

Summary for Reach 1R: POI#1

Inflow Area	a =	1.370 ac, 5	8.86% Imp	ervious,	Inflow De	epth =	3.32	2" for 25-	year event
Inflow	=	4.26 cfs @	12.02 hrs,	Volume	=	0.379 a	af		-
Outflow	=	4.26 cfs @	12.02 hrs,	Volume	=	0.379 a	af, <i>i</i>	Atten= 0%,	Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.10 hrs



Summary for Subcatchment 10S: BUILDING AND PAVEMENT

Runoff = 2.51 cfs @ 12.02 hrs, Volume= 0.212 af, Depth= 2.67"

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-48.00 hrs, dt= 0.10 hrs Type III 24-hr 2-year Rainfall=3.20"

	A	rea (sf)	CN	Description						
		35,160	98	Paved parking, HSG A						
		2,166	98	Water Surface, HSG A						
_		4,213	39	>75% Gras	s cover, Go	bod, HSG A				
		41,539		Weighted Average						
		4,213		10.14% Pervious Area						
		37,326	89.86% Impervious Area							
	Тс	Length	Slope	e Velocity	Capacity	Description				
_	(min)	(feet)	(ft/ft) (ft/sec)	(cfs)					
	5.0					Direct Entry,				
						-				

Summary for Subcatchment 11S:

Runoff = 1.47 cfs @ 12.02 hrs, Volume= 0.124 af, Depth= 1.69"

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-48.00 hrs, dt= 0.10 hrs Type III 24-hr 2-year Rainfall=3.20"

A	rea (sf)	CN	Description						
	16,532	39	>75% Grass cover, Good, HSG A						
	21,787	98	Paved parking, HSG C						
	38,319		Weighted Average						
	16,532		43.14% Pervious Area						
	21,787 56.86% Impervious Area				rea				
Tc (min)	Length (feet)	Slope (ft/ft	e Velocity) (ft/sec)	Capacity (cfs)	Description				
5.0					Direct Entry,				

Summary for Subcatchment 12S:

Runoff = 0.63 cfs @ 12.02 hrs, Volume= 0.053 af, Depth= 0.83"

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-48.00 hrs, dt= 0.10 hrs Type III 24-hr 2-year Rainfall=3.20"

Area (sf)	CN	Description			
24,040	39	>75% Grass cover, Good, HSG A			
9,304	98	aved parking, HSG A			
33,344		Weighted Average			
24,040 72.10% Pervious Area		72.10% Pervious Area			
9,304		27.90% Impervious Area			

Тс	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	-
5.0					Direct Entry,

Summary for Reach 10R: POI#1

Inflow Area	ı =	1.833 ac, 7	74.02% Impe	ervious,	Inflow	Depth =	2.2	0" for	2-ye	ear ev	ent
Inflow	=	1.62 cfs @	12.02 hrs,	Volume	=	0.336	af				
Outflow	=	1.62 cfs @	12.02 hrs,	Volume	=	0.336	af,	Atten= (0%,	Lag=	0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.10 hrs

Summary for Pond 11P: GRASSED UNDERDRAINED SOIL FILTER

Inflow Area	=	0.954 ac,	89.86% Impe	ervious,	Inflow	Depth =	2.67"	for 2-ye	ar event	
Inflow	=	2.51 cfs @	12.02 hrs,	Volume	=	0.212	af	-		
Outflow	=	0.18 cfs @	13.35 hrs,	Volume	=	0.212	af, Atte	en= 93%,	Lag= 80.0	min
Primary	=	0.18 cfs @	13.35 hrs,	Volume	=	0.212	af		-	

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.10 hrs Peak Elev= 294.49' @ 13.35 hrs Surf.Area= 3,216 sf Storage= 3,991 cf

Plug-Flow detention time= 198.9 min calculated for 0.211 af (100% of inflow) Center-of-Mass det. time= 198.6 min (951.0 - 752.5)

Volume	Inve	ert Avail.Sto	rage	Storage Description					
#1 293.00		0' 7,62	29 cf	Custom S	Stage Data (P	rismatic)Listed below (Recalc)			
Elevatio	on	Surf.Area	Inc.	Store	Cum.Store				
(fee	et)	(sq-ft)	(cubic	-feet)	(cubic-feet)				
293.0	00	2,166		0	0				
294.0	00	2,845		2,506	2,506				
295.00 3,601		;	3,223	5,729					
295.50 4,		4,000		1,900	7,629				
Device	Routing	outing Invert		t Devices					
#1	Device 3	293.00'	2.410	2.410 in/hr Exfiltration over Surface area					
#2	Device 3	294.50'	24.0" Horiz. Orifice/Grate C= 0.600						
			Limited to weir flow at low heads						
#3	Primary	290.40'	18.0'	' Round C	Culvert				
			L= 47	7.0' CPP,	projecting, no	headwall, Ke= 0.900			
			Inlet	Inlet / Outlet Invert= 290.40' / 290.20' S= 0.0043 '/' Cc= 0.900					
			n= 0.	013 Corru	igated PE, sm	ooth interior, Flow Area= 1.77 st			

Primary OutFlow Max=0.18 cfs @ 13.35 hrs HW=294.49' (Free Discharge)

-3=Culvert (Passes 0.18 cfs of 12.28 cfs potential flow)

1=Exfiltration (Exfiltration Controls 0.18 cfs)

-2=Orifice/Grate (Controls 0.00 cfs)

Summary for Subcatchment 10S: BUILDING AND PAVEMENT

Runoff = 3.64 cfs @ 12.02 hrs, Volume= 0.313 af, Depth= 3.93"

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-48.00 hrs, dt= 0.10 hrs Type III 24-hr 10-year Rainfall=4.60"

_	A	rea (sf)	CN	Description						
		35,160	98	Paved parking, HSG A						
		2,166	98	Water Surface, HSG A						
		4,213	39	>75% Gras	s cover, Go	ood, HSG A				
_		41,539		Weighted Average						
		4,213		10.14% Pervious Area						
		37,326	,326 89.86% Impervious Area							
	Tc	Length	Slope	e Velocity	Capacity	Description				
_	(min)	(feet)	(ft/ft) (ft/sec)	(cfs)					
	5.0					Direct Entry,				
						•				

Summary for Subcatchment 11S:

Runoff = 2.12 cfs @ 12.02 hrs, Volume= 0.186 af, Depth= 2.54"

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-48.00 hrs, dt= 0.10 hrs Type III 24-hr 10-year Rainfall=4.60"

A	rea (sf)	CN	Description						
	16,532	39	>75% Grass cover, Good, HSG A						
	21,787	98	Paved park	Paved parking, HSG C					
	38,319		Weighted Average						
	16,532	43.14% Pervious Area							
	21,787 56.86% Impervious Are			pervious Are	rea				
Tc (min)	Length (feet)	Slope (ft/ft	velocity (ft/sec)	Capacity (cfs)	Description				
5.0					Direct Entry,				

Summary for Subcatchment 12S:

D 11			10.001		0 000 f		
Runoff	=	0.91 cts (a)	12.02 hrs,	Volume=	0.083 af,	Depth= 1.31"	

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-48.00 hrs, dt= 0.10 hrs Type III 24-hr 10-year Rainfall=4.60"

Area (sf)	CN	Description			
24,040	39	>75% Grass cover, Good, HSG A			
9,304	98	Paved parking, HSG A			
33,344		Weighted Average			
24,040		72.10% Pervious Area			
9,304		27.90% Impervious Area			
Тс	Length	Slope	Velocity	Capacity	Description
-------	--------	---------	----------	----------	---------------
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
5.0					Direct Entry,

Summary for Reach 10R: POI#1

Inflow Area	ı =	1.833 ac, 7	74.02% Impe	ervious,	Inflow	Depth =	3.2	6" for	10-y	year e	vent
Inflow	=	2.81 cfs @	12.20 hrs,	Volume	=	0.499	af				
Outflow	=	2.81 cfs @	12.20 hrs,	Volume	=	0.499	af,	Atten= ()% ,	Lag=	0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.10 hrs

Summary for Pond 11P: GRASSED UNDERDRAINED SOIL FILTER

Inflow Area	=	0.954 ac, 8	39.86% Impe	ervious,	Inflow Depth =	3.93"	for 10	-year ever	nt
Inflow	=	3.64 cfs @	12.02 hrs,	Volume	= 0.313	af		-	
Outflow	=	1.89 cfs @	12.23 hrs,	Volume	= 0.313	af, Atte	en= 48%	6, Lag= 12	2.8 min
Primary	=	1.89 cfs @	12.23 hrs,	Volume	= 0.313	af		-	

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.10 hrs Peak Elev= 294.69' @ 12.23 hrs Surf.Area= 3,368 sf Storage= 4,654 cf

Plug-Flow detention time= 170.3 min calculated for 0.312 af (100% of inflow) Center-of-Mass det. time= 170.2 min (916.7 - 746.5)

Volume	Inve	ert Avail.Sto	rage	Storage D	escription	
#1	293.0	0' 7,62	29 cf	Custom S	Stage Data (P	rismatic)Listed below (Recalc)
Elevatio	on	Surf.Area	Inc.	Store	Cum.Store	
(fee	et)	(sq-ft)	(cubic	-feet)	(cubic-feet)	
293.0	00	2,166		0	0	
294.0	00	2,845		2,506	2,506	
295.0	00	3,601	;	3,223	5,729	
295.	50	4,000		1,900	7,629	
Device	Routing	Invert	Outle	t Devices		
#1	Device 3	293.00'	2.410) in/hr Exf	iltration over	Surface area
#2	Device 3	294.50'	24.0"	' Horiz. Or	ifice/Grate	C= 0.600
			Limite	ed to weir t	flow at low hea	ads
#3	Primary	290.40'	18.0'	' Round C	Culvert	
			L= 47	7.0' CPP,	projecting, no	headwall, Ke= 0.900
			Inlet	/ Outlet Inv	/ert= 290.40' /	290.20' S= 0.0043 '/' Cc= 0.900
			n= 0.	013 Corru	igated PE, sm	ooth interior, Flow Area= 1.77 st

Primary OutFlow Max=1.74 cfs @ 12.23 hrs HW=294.68' (Free Discharge)

-3=Culvert (Passes 1.74 cfs of 12.62 cfs potential flow)

1=Exfiltration (Exfiltration Controls 0.19 cfs)

-2=Orifice/Grate (Weir Controls 1.56 cfs @ 1.38 fps)

Summary for Subcatchment 10S: BUILDING AND PAVEMENT

Runoff = 4.52 cfs @ 12.02 hrs, Volume= 0.393 af, Depth= 4.95"

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-48.00 hrs, dt= 0.10 hrs Type III 24-hr 25-year Rainfall=5.70"

a (sf)	CN I	Description							
5,160	98 I	Paved parking, HSG A							
2,166	98 \	Water Surface, HSG A							
4,213	39 >	>75% Grass cover, Good, HSG A							
1,539	١	Veighted A	verage						
4,213		10.14% Pervious Area							
7,326	89.86% Impervious Area								
ength	Slope	Velocity	Capacity	Description					
(feet)	(ft/ft)	(ft/sec)	(cfs)						
				Direct Entry,					
				•					
	a (sf) 5,160 2,166 4,213 1,539 4,213 7,326 -ength (feet)	a (sf) CN E 5,160 98 F 2,166 98 V 4,213 39 > 1,539 V 4,213 1 7,326 8 _ength Slope (feet) (ft/ft)	a (sf) CN Description 5,160 98 Paved park 2,166 98 Water Surfa 4,213 39 >75% Grass 1,539 Weighted A 4,213 10.14% Per 7,326 89.86% Imp Length Slope Velocity (feet) (ft/ft) (ft/sec)	a (sf)CNDescription5,16098Paved parking, HSG A2,16698Water Surface, HSG A4,21339>75% Grass cover, Go1,539Weighted Average4,21310.14% Pervious Area7,32689.86% Impervious Ar.engthSlopeVelocity(feet)(ft/ft)(ft/sec)(cfs)					

Summary for Subcatchment 11S:

Runoff = 2.64 cfs @ 12.02 hrs, Volume= 0.239 af, Depth= 3.26"

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-48.00 hrs, dt= 0.10 hrs Type III 24-hr 25-year Rainfall=5.70"

A	rea (sf)	CN	Description							
	16,532	39	>75% Grass cover, Good, HSG A							
	21,787	98	Paved parking, HSG C							
	38,319		Weighted A	verage						
	16,532		43.14% Pervious Area							
	21,787		56.86% Imp	pervious Are	rea					
Tc (min)	Length (feet)	Slope (ft/ft	velocity (ft/sec)	Capacity (cfs)	Description					
5.0					Direct Entry,					

Summary for Subcatchment 12S:

Runoff =	1.13 cfs @	12.02 hrs. Volume=	0.114 af. Depth= 1.79"

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-48.00 hrs, dt= 0.10 hrs Type III 24-hr 25-year Rainfall=5.70"

Area (sf)	CN	Description
24,040	39	>75% Grass cover, Good, HSG A
9,304	98	Paved parking, HSG A
33,344		Weighted Average
24,040		72.10% Pervious Area
9,304		27.90% Impervious Area

Тс	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
5.0					Direct Entry,

Summary for Reach 10R: POI#1

Inflow Area	a =	1.833 ac, 7	74.02% Impe	ervious,	Inflow	Depth =	4.14	4" for 25	5-year e	vent
Inflow	=	5.45 cfs @	12.11 hrs,	Volume	=	0.632	af			
Outflow	=	5.45 cfs @	12.11 hrs,	Volume	=	0.632	af, <i>i</i>	Atten= 0%	, Lag=	0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.10 hrs

Summary for Pond 11P: GRASSED UNDERDRAINED SOIL FILTER

Inflow Area	=	0.954 ac, 8	39.86% Impe	ervious,	Inflow Depth	= 4.95	5" for 2	5-year event	
Inflow	=	4.52 cfs @	12.02 hrs,	Volume	= 0.39	93 af		-	
Outflow	=	3.59 cfs @	12.13 hrs,	Volume	= 0.39	93 af, <i>A</i>	Atten= 20	%, Lag= 6.8	8 min
Primary	=	3.59 cfs @	12.13 hrs,	Volume	= 0.39	93 af		-	

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.10 hrs Peak Elev= 294.80' @ 12.13 hrs Surf.Area= 3,453 sf Storage= 5,040 cf

Plug-Flow detention time= 153.6 min calculated for 0.393 af (100% of inflow) Center-of-Mass det. time= 153.5 min (897.1 - 743.7)

Volume	Inve	rt Avail.Sto	rage	ge Storage Description					
#1	293.0	0' 7,62	29 cf	Custom S	tage Data (Pi	ismatic)Listed below (Recalc)			
Elevatio (fee	on et)	Surf.Area (sq-ft)	Inc. (cubic	Store -feet)	Cum.Store (cubic-feet)				
293.0	00	2,166		0	0				
294.0	00	2,845		2,506	2,506				
295.0	00	3,601		3,223	5,729				
295.5	50	4,000		1,900	7,629				
Device	Routing	Invert	Outle	t Devices					
#1	Device 3	293.00'	2.410	2.410 in/hr Exfiltration over Surface area					
#2	Device 3	294.50'	24.0"	' Horiz. Or	ifice/Grate	c= 0.600			
			Limite	ed to weir f	low at low hea	ads			
#3	Primary	290.40'	18.0"	' Round C	ulvert				
			L= 47 Inlet / n= 0.	L= 47.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 290.40' / 290.20' S= 0.0043 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.77 sf					

Primary OutFlow Max=3.25 cfs @ 12.13 hrs HW=294.78' (Free Discharge)

3=Culvert (Passes 3.25 cfs of 12.80 cfs potential flow)

1=Exfiltration (Exfiltration Controls 0.19 cfs)

-2=Orifice/Grate (Weir Controls 3.06 cfs @ 1.73 fps)









LEGEND:



STORMWATER POND SUBCATCHMENT AREA POINT OF INTEREST



		1. 06-19-2019 SUBMITTED TO 1	TOWN FOR FINAL REVIEW	JJM
		POST-I	DEVELOPMENT	
		WATE	ERSHED PLAN	
		PROJECT: CONVENIENCE S 134 MAIN STREET,	TORE AND OFFICE BL EAST WATERBORO, M	IILDING 1E 04030
		PREPARED FOR: NEW HORIZONS N 3391 WHITE SULPHUR	IANAGEMENT COMPAI R ROAD, GAINESVILLE	NY, LLC , GA 30501
60	JOSEPH		SITELI 119 PURINTON RO BRUNSWICK, MAI 207.725.12 PLANNERS • LAND SU	NES AD, SUITE A NE 04011 200 RVEYORS
	MARDEN 12828	FIELD WK: OTHERS	SCALE: 1"=30'	SHEET:
	LICENSED G	DRN BY: JJM	JOB #: 3841	1 D 2
	ONAL	CH'D BY: CYN	MAP/LOT: 5/47C&48-1	$ D_{\perp} $
	//~ 06-19-19	DATE: 03-25-19	FILE: 3841-SITE	

Site Plan Review Final Application Convenience Store and Office Building June 19, 2019

Attachment H Lighting Details

Information on the proposed lighting is enclosed for reference.



LING Series SI ENIDER WALLBACK

FEATURES

- · Two sizes for a variety of applications
- Ranges from 10W to 80W with up to 8000 lumens
- SG1 Series replaces from 100W-150W HID; SG2 Series replaces from 150W-250W HID
- · Comfort lens available as an option or accessory provides glare control and enhanced uniformity
- · Knuckle and trunnion accessory mounting kits available for flood applications
- IP65 and certified to UL 1598 for use in wet locations up to 40°C ambient
- DLC (DesignLights Consortium Qualified see www.designlights.org



DATE:	LOCATION:
TYPE:	PROJECT:
CATALOG #:	

tradeSELECT





RELATED PRODUCTS								
8 LNC Litepak	8 LNC2 Litepak	8 LNC3 Lite						
8 LNC4 Litepak	8 GeoPak	8 <u>GeoPak</u> 2						

SPECIFICATIONS

HOUSING

- Rugged die-cast aluminum housing with corrosion resistant powder coat finish
- · Heating dissipating fins provide superior thermal performance extending the life of the electronic components
- Impact resistant tempered glass offers zero uplight
- Comfort lens available as an option or accessory to reduce glare (7-10% lumen reduction) and provide better uniformity

OPTICS

- 3000K, 4000K and 5000K CCT nominal with 70 CRI
- Smaller SG1 housing has 2 LEDs, larger SG2 housing has 3 LEDs

INSTALLATION

- · Side hinge allows for easy installation and wiring
- · Side movement avoids damage to the lens and helps prevent injury common in drop down hinge designs
- · Mounts to 4" junction box and includes a gasket to help seal electrical connections
- Four 1/2" threaded conduits hubs for surface conduit provided

ELECTRICAL

- 120-277V, 50/60Hz electronic drivers
- 347V and 480V available in large SG2 housing
- 10KA surge protection included

OPTIONS/CONTROLS

- Button photocontrol for dusk to dawn energy savings. Stock versions include 120V-277V PC with a cover which provides a choice to engage photocontrol or not. PC is installed in top hub
- Occupancy sensor available for on/off and dimming control in larger SG2 housing
- SiteSync[™] wireless lighting control delivers flexible control strategies for reducing power consumption and minimizing maintenance costs while delivering the right light levels with a simple and affordable wireless solution.
- · See ordering information or visit www.hubbelllighting.com/sitesync for more details
- · Battery backup options available in larger SG2 housing rated for either 0° C or -30° C. Performance exceeds NEC requirement providing 1 fc minimum over 10'x10' at 11' mounting height

CERTIFICATIONS

- DesignLights Consortium® (DLC) qualified. Please refer to the DLC website for specific product qualifications at www.designlights.org
- Listed to UL1598 for use in wet location, listed for -40°C to 40°C applications
- IDA approved with zero uplight for 3000K and warmer CCTs

• IP65

- WARRANTY 5 year limited warranty
- · See HLI Standard Warranty for additional information

KEY DATA	A
Lumen Range	2263-8079
Wattage Range	21–80
Efficacy Range (LPW)	101–113
Fixture Projected Life (Hours)	L70>50K
Weights lbs. (kg)	4.3–11 (2.0–5.0)



8 PHOTOMETRY

8 SG2 PSG PAGE



SLING SERIES

SLENDER WALLPACK

ORDERING GUIDE

 DATE:
 LOCATION:

 TYPE:
 PROJECT:

 CATALOG #:
 CATALOG #:

Example: SG1-20-3K7-FT-UNV-DB-PCU-CS

CATALOG #

ORDERING INFORMATION

		-			_			-			-			-			_		
Housing			CCT	/CRI		Dis	tribution		Volta	ge		Colo	r/Finish		Control C	Options		Optio	ns
SG1-10	Size 1, 10W		3K7	3000K,		FT	Fwd Throw		UNV	120V-277V		DB	Textured Dark		PCU	Button Photocontrol (120-277V)		CS	Comfort Lens
SG1-20	Size 1, 20W			70 CRI					120	120V			Bronze		SCP ^{1,2,3}	Hi-Lume 1% 3-wire /		E ^{1,2}	Batter 0°C
SG1-30	Size 1, 30W		4K7	4000K, 70 CRI					277	277V		BL	Textured Black		0141512	EcoSystem LED Driver		EH ^{1,2}	Battery w/ heater
SG1-40	Size 1, 40W		5K7	5000K					UHV	347V-480V		WH	lextured White		SWP	Hi-Lume 1% 2-wire LED Driver			-20°C
SG2-50	Size 2, 50W		51(7	70 CRI								PS	Smooth Gray Smooth Plat. Silver		SWPM ^{1,2}	Osram dimming to 1% 0–10V			
SG2-80	Size 2, 80W											cc	Custom Color		Specify N	ITG HT for SCO/SCP & SWPM			
															8F	Up to 8'			
															20F	Up to 20'			

Notes:

1 Available in SG2 only, UHV available in SG2-50 only

2 Sensor controls & battery backup can not be used with flood accessory or kit or for inverted/up mounting, 120-227V only for SCO/SCP, 120 or 277 only for SWP, SWPM, E & EH

3 Must order minimum of one remote control to program dimming settings, 0-10V fully adjustable dimming with automatic daylight calibration and different time delay settings, 120-277V only

STOCK ORDERING INFORMATION

Catalog Number	CCT/CRI	Wattage	Mounting Height	Color	Color	Delivered Lumens	LPW	Weight Ibs. (kg)
SG1-10-PCU	5000K/70	11W	8–12ft	120-277V	Dark Bronze	1349	122	4.3 (2.0)
SG1-10-4K-PCU	4000K/70	11W	8–12ft	120–277V	Dark Bronze	1424	129	4.3 (2.0)
SG1-20-PCU	5000K/70	21W	8–12ft	120-277V	Dark Bronze	2263	108	4.3 (2.0)
SG1-20-4K-PCU	4000K/70	21W	8–12ft	120-277V	Dark Bronze	2310	110	4.3 (2.0)
SG1-30-PCU	5000K/70	29W	10–15ft	120–277V	Dark Bronze	3270	113	4.3 (2.0)
SG1-30-4K-PCU	4000K/70	29W	10–15ft	120-277V	Dark Bronze	3060	105	4.3 (2.0)
SG1-40-PCU	5000K/70	38W	10–15ft	120–277V	Dark Bronze	4008	105	4.3 (2.0)
SG1-40-4K-PCU	4000K/70	38W	10–15ft	120–277V	Dark Bronze	4070	106	4.3 (2.0)
SG2-50-PCU	5000K/70	51W	12–18ft	120–277V	Dark Bronze	5548	110	11 (5.0)
SG2-50-4K-PCU	4000K/70	51W	12–18ft	120-277V	Dark Bronze	5526	109	11 (5.0)
SG2-80-PCU	5000K/70	80W	15–25ft	120–277V	Dark Bronze	8061	101	11 (5.0)
SG2-80-4K-PCU	4000K/70	80W	15–25ft	120-277V	Dark Bronze	8079	101	11 (5.0)





SLING SERIES

TYPE: PROJECT: CATALOG #:

LOCATION:

DATE:

ORDERING GUIDE

OPTIONS AND ACCESSORIES

Catalog Number	Description	Weight Ibs. (kg)
SG1-CS	Acrylic comfort lens for SG1	1 (.45)
SG2-CS	Acrylic comfort lens for SG2	1 (.45)
SG1-YOKE	SG1 Series Yoke/Floodlight mount kit, includes visor	2.0 (1.0)
SG1-KNUCKLE	SG1 Series Knuckle/Floodlight mount kit, includes visor	2.0 (1.0)
SG2-YOKE	SG2 Series Yoke/Floodlight mount kit, includes visor	2.0 (1.0)
SG2-KNUCKLE	SG2 Series Knuckle/Floodlight mount kit, includes visor	2.0 (1.0)
SCP-REMOTE*	Remote control for SCP option. Order at least one per project to program and control fixtures	1 (.45)
SG2-PMA-3-XX	3" Pole Mount adapter for SG2, both square and round, XX= finish	7 (3.5)
SG2-PMA-4-XX	4" Pole Mount adapter for SG2, both square and round, XX= finish	7 (3.5)
SG2-PMA-5-XX	5" Pole Mount adapter for SG2, both square and round, XX= finish	7 (3.5)
SG2-PMA-6-XX	6" Pole Mount adapter for SG2, both square and round, XX= finish	7 (3.5)
SG1-SPC	Vandal Resistant Lens (shield polycarbonate), SG1	3 (1.5)
SG2-SPC	Vandal Resistant Lens (shield polycarbonate), SG2	3 (1.5)
SG1-WCP	Universal Wall Cover Plate, Dark Bronze, SG1	10 (5)
SG2-WCP-H	Horizontal Mount Wall Cover Plate, Dark Bronze, SG2	10 (5)
SG2-WCP-V	Vertical Mount Wall Cover Plate, Dark Bronze, SG2	10 (5)
SG2XL-WCP-H	Horizontal Mount Wall Cover Plate, DB, SG2 with battery or sensor	10 (5)
SG2XL-WCP-V	Vertical Mount Wall Cover Plate, DB, SG2 with battery or sensor	10 (5)

ACCESSORIES AND SERVICES (ORDERED SEPARATELY)

Control Option	IS
SWUSB ¹	SiteSync [™] interface software loaded on USB flash drive for use with owner supplied PC (Windows based only). Includes SiteSync [™] license, software and USB radio bridge node.
SWTAB ¹	Windows tablet and SiteSync [™] interface software. Includes tablet with preloaded software, SiteSync [™] license and USB radio bridge node.
SWBRG ²	SiteSync [™] USB radio bridge node only. Order if a replacement is required or if an extra bridge node is requested.

Notes:

1~ When ordering SiteSync $^{\bowtie}$ at least one of these two interface options must be ordered per project.

2 If needed, an additional Bridge Node can be ordered.





SLING SERIES

SLENDER WALLPACK

DATE:	LOCATION:
TYPE:	PROJECT:

PERFORMANCE DATA

Description	System	5K (500	OK NO	MINA	L 70 C	:RI)	4K (400	OK NO	3K (3000K NOMINAL 80 CRI)									
Description	LEDs	Current	Watts	Lumens	LPW	В	U	G	Lumens	LPW	В	U	G	Lumens	LPW	В	υ	G
SG1-10	2	140mA	11	1349	122	1	0	0	1424	129	1	0	0	1003	91	1	0	0
SG1-20	2	250mA	21	2449	115	1	0	0	2310	110	1	0	0	2054	95	1	0	0
SG1-30	2	350mA	29	3332	117	2	0	0	3060	106	1	0	0	2913	100	1	0	0
SG-40	2	450mA	38	4008	105	2	0	0	4070	106	2	0	0	3845	100	2	0	0
SG2-50-UHV	3	350mA	44	4633	106	2	0	0	4609	105	2	0	0	3895	90	2	0	0
SG2-50	3	415mA	51	5548	109	2	0	0	5526	107	2	0	0	4700	92	2	0	0
SG2-80	3	650mA	80	7851	98	2	0	1	8079	103	2	0	1	6721	86	2	0	1

CATALOG #:

*347 and 480 VAC input Lumen values are from photometric test performed in accordance with IESNA LM-79-08. Data is considered to be representative of the configurations shown. Actual performance may differ as a result of end-user environment application and inherent performance balances of the electrical components.

ELECTRICAL DATA

Catalog number	# of Drivers	Input Voltage	Current (AMPS)	System Power
561.10	1	120	0.09	11.0
561-10	1	277	0.04	11.0
561.20	1	120	0.18	21.0
5GI-20	1	277	0.08	21.0
501 20	1	120	0.24	28.9
361-30	1	277	0.10	28.9
562.40	1	120	0.32	38.3
5G2-40	1	277	0.14	38.3
	1	347	0.13	43.5
SG-50-UHV	1	480	0.18	43.5
5C2 F0	1	120	0.42	50.6
362-50	1	277	0.18	50.6
563.80	1	120	0.68	79.8
362-80	1	277	0.29	79.8

PROJECTED LUMEN MAINTENANCE

Ambient Temperature	OPERATING HOURS					
	0	25,000	50,000	TM-21-11 ¹ L96 60,000	100,000	L70 (Hours)
25°C / 77°F	1.00	0.98	0.97	0.96	0.95	>791,000
40°C / 104°F	0.99	0.98	0.96	0.96	0.94	>635,000

1. Projected per IESNA TM-21-11 * (Nichia 219B, 700mA, 85°C Ts, 10,000hrs) Data references the extrapolated performance projections for the base model in a 40°C ambient, based on 10,000 hours of LED testing per IESNA LM-80-08





SLI	NG	SE	R	ES
SLEND	ER WAL	LPACK		

DATE:	LOCATION:
TYPE:	PROJECT:
CATALOG #:	

LUMINAIRE AMBIENT TEMPERATURE FACTOR (LATF)

Ambient Te	Lumen Multiplier	
0° C	32° F	1.02
10° C	50° F	1.01
20° C	68° F	1.00
25° C	77° F	1.00
30° C	86° F	1.00
40° C	104° F	0.99
50° C	122° F	0.96

Use these factors to determine relative lumen output for average ambient temperatures from 0-40°C (32-104°F).

DIMENSIONS





А	В	с	Weight
4.19"	7.80"	6.61"	4.4lbs
(107mm)	(198mm)	(168mm)	(2kg)





А	В	С	Weight
5.80" (147mm)	11.14" (283mm)	9.52" (242mm)	111bs (5kg)

SG2 with occupancy sensor and battery options



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SLING SERIES SLENDER WALLPACK

PHOTOMETRY

SG1-10-4K7

LUMINAIRE DATA

Description	4000 Kelvin, 70 CRI
Distribution Type	Forward Throw
Delivered Lumens	1424
Watts	11.4
Efficacy	125
Mounting	Wall

ZONAL LUMEN SUMMARY

Zone	Lumens	% Luminaire
Downward Street Side	996.6	70.0
Downward House Side	427.8	30.0
Downward Total	1424.4	100.0
Upward Street Side	0.0	0.0
Upward House Side	0.0	0.0
Upward Total	0.0	0.0
Total Flux	1424.4	100.0



SG1-20-4K7

LUMINAIRE DATA

Description	4000 Kelvin, 70 CRI
Distribution Type	Foward Throw
Delivered Lumens	2310
Watts	20.9
Efficacy	111
Mounting	Wall

ZONAL LUMEN SUMMARY

Zone	Lumens	% Luminaire
Downward Street Side	1618	70.0
Downward House Side	692.1	30
Downward Total	2310	100.0
Upward Street Side	0.0	0.0
Upward House Side	0.0	0.0
Upward Total	0.0	0.0
Total Flux	2310.3	100.0

ISOMETRIC FOOTCANDLE



SG1-30

LUMINAIRE DATA

Description	4000 Kelvin, 70 CRI
Distribution Type	Forward Throw
Delivered Lumens	3060
Watts	29.1
Efficacy	105
Mounting	Wall

ZONAL LUMEN SUMMARY

Zone	Lumens	% Luminaire
Downward Street Side	2619.4	70.9
Downward House Side	890.4	29.1
Downward Total	3059.8	100.0
Upward Street Side	0.0	0.0
Upward House Side	0.0	0.0
Upward Total	0.0	0.0
Total Flux	3059.8	100.0

ISOMETRIC FOOTCANDLE



DATE:	LOCATION:
TYPE:	PROJECT:
CATALOG #:	





SLING SERIES SLENDER WALLPACK

PHOTOMETRY

SG1-40-4K7

LUMINAIRE DATA

Description	4000 Kelvin, 70 CRI
Distribution Type	Foward Throw
Delivered Lumens	4070
Watts	38.1
Efficacy	107
Mounting	Wall

ZONAL LUMEN SUMMARY

Zone	Lumens	% Luminaire
Downward Street Side	2857.7	70.2
Downward House Side	1215.5	29.8
Downward Total	4070.2	100.0
Upward Street Side	0.0	0.0
Upward House Side	0.0	0.0
Upward Total	0.0	0.0
Total Flux	4070.2	100.0



SG2-50-4K7

LUMINAIRE DATA

Description	4000 Kelvin, 70 CRI
Distribution Type	Foward Throw
Delivered Lumens	5525.7
Watts	51.7
Efficacy	107
Mounting	Wall

ZONAL LUMEN SUMMARY

Zone	Lumens	% Luminaire
Downward Street Side	4611.8	83.5
Downward House Side	913.9	16.5
Downward Total	5525.7	100.0
Upward Street Side	0.0	0.0
Upward House Side	0.0	0.0
Upward Total	0.0	0.0
Total Flux	5525.7	100.0

ISOMETRIC FOOTCANDLE



SG2-80-4K7

LUMINAIRE DATA

Description	4000 Kelvin, 70 CRI
Distribution Type	Foward Throw
Delivered Lumens	8453
Watts	78.5
Efficacy	108
Mounting	Wall

ZONAL LUMEN SUMMARY

Zone	Lumens	% Luminaire
Downward Street Side	6677.7	79.0
Downward House Side	1775.5	21.0
Downward Total	8453.2	100.0
Upward Street Side	0.0	0.0
Upward House Side	0.0	0.0
Upward Total	0.0	0.0
Total Flux	8453.2	100.0

ISOMETRIC FOOTCANDLE



DATE:	LOCATION:
TYPE:	PROJECT:
CATALOG #:	

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AIROLED-SPEC	





SLING SERIES SLENDER WALLPACK

DATE:	LOCATION:
TYPE:	PROJECT:
CATALOG #:	

ADDITIONAL INFORMATION

Shipping Information

Catalog Number		Carton Dimensions			Carton Qty.	
	CTN	Length Inch (cm)	Width Inch (cm)	Height Inch (cm)	per Master Pack	Pallet Qty.
SG1	4.35lbs (2kg)	9.5 (24)	8.25 (21)	5.25 (13)	6	98
SG2	11lbs (5kg)	14 (36)	11.5 (29)	8 (20)	2	64

Accessories and Services





Visor



Mounting Options

Acrylic comfort lens provides glare control, improved visual comfort and better uniformity

Visor accessory accessory kits

Flood mounting accessories - 1/2" threaded included with mounting knuckle or yoke (includes grommet and 3' SO cord)



Photocontrol option available for energysaving dusk-to-dawn operation

Hinged Housing Door



Side hinged for easy installation and wiring access, single screw secures housing closure









SG(X)-WCP-H



SG(X)-WCP-V

Features



Battery back up feature with side indicator.

Exceeds Life Safety Code average illuminance of 1.0 fc. at 12' mounting height. Assumes open space with no obstructions. Battery backup units consume 6W when charging a dead battery and 2W during maintenance charging. EH (units with a heater) consume up to an additional 8W when charging if the battery temp is lower than 10°C

Diagrams for illustration purposes only, please consult factory for application layout.





SiteSync[™] Lighting Control delivers flexible control strategies for reducing power consumption and minimizing maintenance costs while delivering the right light levels with a simple and affordable wireless solution.









l	Cat.#
ſ	Job

Туре



Approvals

SPECIFICATIONS Intended Use

The Beacon Viper luminaire is available in two sizes with a wide choice of different LED wattage configurations and optical distributions designed to replace HID lighting up to 1000W MH or HPS. Luminaires are suitable for wet locations.

Construction:

- Manufactured with die cast aluminum.
- Coated with a polyester finish that meets ASTM B117 corrosion test requirements and ASTM D522 cracking and loss of adhesion test requirements.
- External hardware is corrosion resistant. One piece optical cartridge system consisting of an LED engine, LED lamps, optics, gasket
- and stainless steel bezel. Cartridge is held together with internal brass standoffs soldered to the board so that it can be field replaced as a one piece optical system.
- Two-piece silicone and microcellular polyurethane foam gasket ensures a weather-proof seal around each individual optic.

Electrical:

- Luminaire accepts 100V through 277V, 50 Hz to 60 Hz (UNV), 347V, or 480V input.
- Power factor is ≥ .90 at full load.
- All electrical components are rated at 50,000 hours at full load and 25°C ambient conditions per MIL- 217F Notice 2.
- Dimming drivers are standard, but must contact factory to request wiring leads for purpose of external dimming controls.
- Component-to-component wiring within the luminaire may carry no more than 80% of rated load and is certified by UL for use at 600VAC at 90°C or higher.
- Plug disconnects are certified by UL for use at 600 VAC, 13A or higher. 13A rating applies to primary (AC) side only.
- Fixture electrical compartment shall contain all LED driver components and shall be provided with a push-button terminal block for AC power connections.
- Optional 7-pin ANSI C136.41-2013 twist-lock photo control receptacle available. Compatible with ANSI C136.41 external wireless control devices
- Surge protection 20kA.
- Lifeshield[™] Circuit protects luminaire from excessive temperature. The device shall activate at a specific, factory-preset temperature, and progressively reduce power over a finite temperature range. Operation shall be smooth and undetectable to the eye. Thermal circuit is designed to "fail on", allowing the luminaire to revert to full power in the event of an interruption of its power supply, or faulty wiring connection to the drivers. The device shall be able to co-exist with other 0-10V control devices (occupancy sensors, external dimmers, etc.).

CERTIFICATIONS/LISTINGS



Controls/Options:

- · Available with an optional passive infrared (PIR) motion sensor capable of detecting motion 360° around the luminaire. When no motion is detected for the specified time, the motion response system reduces the wattage to factory preset level, reducing the light level accordingly. When motion is detected by the PIR sensor, the luminaire returns to full wattage and full light output. Please contact Beacon Products if project requirements vary from standard configuration · Available with Energeni for optional set dimming, timed dimming with simple delay, or
- timed dimming based on time of night (see www.beaconproducts.com/products/energeni) • In addition, Viper can be specified
- with SiteSync[™] wireless control system for reduction in energy and maintenance costs while optimizing light quality 24/7. For more details, see ordering information or visit: www.hubbelllighting.com/sitesync

Installation:

· Mounting options for horizontal arm, vertical tenon or traditional arm mounting available. Mounting hardware included.

Finish:

- · IFS polyester powder-coat electrostatically applied and thermocured. IFS finish consists of a five stage pretreatment regimen with a polymer primer sealer and top coated with a thermoset super TGIC polyester powder coat finish
- The finish meets the AAMA 2604 performance specification which includes passing a 3000 hour salt spray test for corrosion resistance and resists cracking or loss of adhesion per ASTM D522 and resists surface impacts of up to 160 inch-pounds.

Certifications/Ratings:

- DesignLights Consortium (DLC) qualified, consult DLC website for more details: http:// www.designlights.org/QPL
- Certified to UL 1598 and CSA C22.2 No. 250.0
- 3G rated for ANSI C136.31 high vibration applications with MAF mounting
- IDA approved

Warranty:

Five year limited warranty for more information visit: www.hubbelllighting.com/resources/warranty



DIMENSIONS



MOUNTING OPTIONS





Side View



Back View



Rectangular Arm (A)





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our most popular brands in a broad range of award-winning product families.

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PERFORMANCE DATA			5K				4K					3K						
			(5000K nominal, 70 CRI)				(4000K nominal, 70 CRI)				(3000K nominal, 70 CRI)							
# LED'S	DRIVE Current (Milliamps)	SYSTEM WATTS	DISTRIBUTION TYPE	LUMENS	LPW ¹	В	U	G	LUMENS	LPW ¹	B	U	G	LUMENS	LPW ¹	В	U	G
			FR	18220	132	2	0	2	18783	137	2	0	2	16341	119	2	0	2
			2	17228	125	2	0	2	17761	129	2	0	2	15452	112	2	0	2
			4	16864	125	2	0	3	17386	129	2	0	3	15125	112	2	0	3
64	625 mA	135W	4W	16797	122	2	0	3	17317	126	2	0	3	15066	109	2	0	3
			5QM	17259	125	4	0	2	17792	129	4	0	2	15479	112	4	0	2
			5QN	18023	131	4	0	0	18580	135	4	0	0	16165	117	4	0	0
			5W	16498	127	4	0	2	17940	124	4	0	3	14797	108	4	0	2
	700 mA		FR	23230	128	2	0	2	23948	132	2	0	2	20835	115	2	0	2
			2	21965	121	3	0	3	22645	125	3	0	3	19701	109	2	0	3
			4	21502	119	2	0	4	22003	123	2	0	4	19734	109	2	0	4
80		180W	4W	21416	118	2	0	4	22079	122	2	0	4	19209	106	2	Ő	4
			5QM	22005	121	4	0	2	22686	125	4	0	2	19736	109	4	0	2
			50	22979	12/	4		1	23689	131	4	0	1	10000	114	4	0	
			5W	21035	116	5	0	3	21686	120	5	0	3	18867	104	4	0	3
		235W	FR	27849	121	2	0	2	28711	125	2	0	2	24978	108	2	0	2
	875 mA		2	26334	114	3	0	3	27148	118	3	0	4	23619	102	3	0	3
			4	25777	114	2	0	4	26575	115	2	0	5	23039	100	2	0	4
80			4W	25675	111	3	0	4	26469	115	3	0	4	23028	100	3	0	4
			5QM	26381	114	4	0	2	27196	118	4	0	2	23661	103	4	0	2
			5UN 5B	27548	119	5		5	28400	123	5	0	5	23868	107	5	0	4
			5W	25218	109	5	0	3	25998	113	5	0	3	22618	98	5	0	3
			FR	27876	128	2	0	2	28738	132	2	0	2	25002	115	2	0	2
			2	26359	121	3	0	3	27174	125	3	0	4	23641	109	3	0	3
	700 mA	220W	4	25802	119	2		4	26600	125	$\frac{3}{2}$	0	4	23081	109	2	0	4
96			4W	25700	118	3	0	4	26494	122	3	0	4	23050	106	3	0	4
			5QM	26406	121	4	0	2	27222	125	4	0	2	23683	109	4	0	2
			5QN	27575	127	5	0	1	28427	131	5	0	1	24732	114	5	0	1
			5W	25242	116	5	0	3	26023	120	5	0	3	22640	104	4	0	3
			FR	33419	121	3	0	2	34453	125	3	0	2	29974	108	2	Ō	2
			2	31600	114	3	0	4	32577	118	3	0	4	28342	102	3	0	4
			3	31654	114	3	0	5	32633	118	3	0	5	28390	103	3	0	4
96	875 mA	280W	4 4W	30810	112	3	0	5	31763	115	3	0	5	27634	100	3	0	4
			5QM	31657	114	5	0	3	32636	118	5	0	3	28393	103	4	0	2
			5QN	33058	119	5	0	1	34080	123	5	0	1	29650	101	5	0	1
			5W	31933	109	5		5	32921	113	5	0	5	28641	104 08	5	0	3
			FR	35666	113	3	0	2	36769	117	3	0	2	31989	101	2	0	2
96	1000mA	315₩²	2	33725	107	3	0	4	34768	110	3	0	4	30248	96	3	0	4
			3	33782	107	3	0	5	34827	110	3	0	5	30299	96	3	0	4
			4 4W	32882	105	2	0	5	33899	108	2	0	5	29009	<u>94</u> 93	2	0	5
			5QM	33785	107	5	0	3	34830	110	5	0	3	30302	96	5	0	2
			5QN	35280	112	5	0	1	36371	115	5	0	1	31643	100	5	0	1
			5R	34080	108	5	0	5	35134	111	5	0	5	30567	97	5	0	5
	1225mA	395W ²	SW FR	39569	102	3		4	43125	110	3	0	4	37518	<u>92</u> 96	3	0	4
			2	39569	101	3	0	4	40793	104	3	Ŏ	4	35490	91	3	0	4
			3	39619	101	3	0	5	40845	104	3	0	5	35535	91	3	0	5
06			4	38723	98	3		5	39921	101	3		5	34731	88	2	0	5
90			4W	39623	101	5 5	0	3	40848	101	5 5	0	3	35538	<u> </u>	5	0	3
			5QN	41394	105	5	Ő	1	42675	109	5	<u>0</u>	1	37127	95	5	0	1
			5R	39969	102	5	0	5	41205	105	5	0	5	35848	91	5	0	5
i I			5W	3/8//	1 97	15	10	14	39048	1 100	15	10	14	33986	87	5	0	4

¹ Lumen values are from photometric tests performed in accordance with IESNA LM-79-08. Data is considered to be representative of the configurations shown. Actual performance may differ as a result of end-user environment and application. ² 315W and 395W 3000K versions are not DLC QPL listed. Reference highlighted cells in table.





PHOTOMETRICS



Type 5W

Type 5QN

Type 5QM

Type 5R









ELECTRICAL DATA

	NUMBER OF	DRIVE CURRENT	INPUT VOLTAGE	SYSTEM POWER	CURRENT
# OF LEDS	DRIVERS	(mA)	(V)	(w)	(Amps)
			120		1.4
64	1	625 mA	277	135	0.6
			480		0.5
			120		1.8
80	2	700 mA	277	180	0.8
00	2		347	100	0.6
			480		0.5
			120		2.4
80	2	875 mA	2//	235	1.0
			347		0.8
			120		2.0
	2	700 mA	277	220	1.0
96			347		0.8
			480		0.6
			120		2.8
06	2	875 mA	277	280	1.2
90			347	200	1.0
			480		0.7
			120		3.2
96	2	1000 mA	2//	315	1.4
			480		0.8
	1		120		4.0
96	2	1225 mA	277	305	1.7
30	<u> </u>	1223 IIIA	347	390	1.4
			480		1.0

PROJECTED LUMEN MAINTENANCE

AMBIENT TEMP.	0	25,000	50,000	'TM-21-11 60,000	100,000	Calculated L70 (HOURS)
25°C / 77°C	1	0.98	0.97	0.97	0.96	>377,000

¹ Projected per IESNA TM-21-11

Data references the extrapolated performance projections for the 700mA base model in a 25°C ambient, based on 10,000 hours of LED testing per IESNA LM-80-08.

Compatible with Pole drill pattern B3 Config. EPA Config. EPA 4" Suggested distance from 3 @ 120° 3.0 1 1.2 top of pole 2.50" Ø5/8"·· 2X Ø5/16"… 2 @ 90° 1.9 3 @ 90° 3.1 Rectangular Arm -Ø4" Pole 2 @ 180° 2.4 4 @ 90° 3.8 -Ø5" Pole -Ø6" Pole

EPA



DRILL PATTERN

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HOUSE SIDE SHIELD FIELD INSTALL ACCESSORIES



HSS/EVP-L/90-FB/XXX 90° shield front or back (2 shields shown)



HSS/EVP-L/270-LR/XXX 270° shield left or right (1 shield shown in right orientation)



HSS/EVP-L/90-LR/XXX 90° shield left or right (1 shield shown in left orientation)



HSS/EVP-L/360/XXX Full shield (1 shield shown)



HSS/EVP-L/270-FB/XXX 270° shield front or back (1 shield shown in back orientation)





Site Plan Review Final Application Convenience Store and Office Building June 19, 2019

Attachment I Architecture

The floor plan and architectural elevations are included here for reference.







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Site Plan Review Final Application Convenience Store and Office Building June 19, 2019

<u>Attachment J</u> <u>Site Plans</u>

The project site plans are included for review as a separate plan set of full-size documents.